

Cosgrave Developments



Blackwood Square, Northwood, Santry Demense, Dublin 9

Planning Application to An Bord Pleanala

Traffic and Transport Assessment

September 2019



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SECTION 1: INTRODUCTION

1.1 Background

J.B. Barry & Partners Ltd. was commissioned by Cosgrave Developments to undertake a Traffic & Transport Assessment (TTA) for a proposed residential development at Blackwood Square, Northwood, Dublin 9.

The proposed development will consist the construction of 4 No.7-storey plus penthouse apartment blocks containing 331 No. apartment units, a multi-function area (c.133sq.m), a gym (c.140sq.m), a childcare facility (c.224sq.m), a concierge (c 81.5sq m) in Block A, 5 No. ground floor mixed use commercial units with a total area of c. 939sq.m; associated car parking (including 334 resident spaces at basement level), 760 No. bicycle storage spaces, 5 No. motorbike spaces, refuse storage, substation, landscaped public open space; network of pedestrian and cycle paths tying in with existing pedestrian and cycle paths on Northwood Avenue with access points along the south, north east and west boundaries of the site; and associated drainage arrangements, landscaping and site development works, all on a site of c. 2.119h.

1.2 Scoping Study

Scoping of the proposal identified the following issues for consideration in the TTA Report:

This TTA provides an assessment of the potential traffic impacts associated with the Blackwood Square development. In this regard, the assessment aims to:

- Identify the existing environment in terms of traffic, existing transport infrastructure and emerging transport developments;
- Quantify the likely vehicle traffic flows to and from the development, from and to the surrounding road network;
- Identify and quantify the likely traffic impacts on the surrounding road network resulting from the development;
- Identify any potential safety issues, in particular impacts on vulnerable road users in the study area;
- Produce a car and bicycle parking strategy;
- Identify suitable measures to mitigate traffic and transportation impacts, if any, associated directly with the development.

The assessment is based on the findings of site visits, traffic observations, on-site traffic counts, architectural plans, consultations with the Design Team and consultation with Fingal County Council.

1.3 Objectives

This report provides an assessment of the potential traffic impacts associated with the proposed development. In this regard, the assessment aims to:

- Identify the existing environment in terms of traffic and transportation;
- Quantify the likely vehicle traffic flows to and from the development from and to the surrounding road network;
- Identify and quantify the likely traffic impacts on the surrounding road network resulting from the development;
- Identify vehicle queueing coming from the junction; and
- Identify suitable measures to mitigate traffic and transportation impacts, if any, associated directly with the development.

The assessment is based on the findings of site visits, traffic observations, on-site traffic counts, architectural plans, and consultations with the Design Team.

1.4 Methodology

The methodology adopted for this report is summarised as follows:

- A site audit was undertaken to quantify existing road network issues and identify local infrastructure characteristics, in addition to establishing the level of accessibility to the Site in terms of walking, cycling and public transport;
- Reference was made to site layout drawings;
- Existing and proposed access arrangements for the proposed development onto Northwood Avenue were considered;
- Traffic surveys were undertaken at the junctions most likely to be impacted by the proposed development;
- A trip generation exercise has been carried out to establish the potential level of vehicle trips generated by the proposed development;
- In accordance with the Traffic and Transport Assessment Guidelines, the specific level of influence generated by the proposed development upon the surrounding local road network was ascertained and the junctions which required assessment in greater detail were identified;
- The junctions considered most likely to be impacted upon by traffic movements associated with the development were assessed in terms of capacity.

In preparing this assessment, reference has been made to the following documents:

- TII “Traffic and Transport Assessment Guidelines” 2014
- Department of Transport, Tourism and Sport “Traffic Management Guidelines” 2012;
- TII Project Appraisal Guidelines: Unit 5.3 - Travel Demand Projections;
- Design Manual for Urban Roads and Streets (DMURS);
- Fingal Development Plan 2017-2023;
- Department of Housing - Design Standards for New Apartments, Guidelines for Planning Authorities;
- Traffic Signs Manual.

SECTION 2: RECEIVING ENVIRONMENT

2.1 Site Location

The proposed development site is located in the Northwood area, in Dublin 9. **Figure 1** and **Figure 2** below illustrates the location and setting of the proposed Blackwood Square development.



Figure 1: Location of Proposed Blackwood Square Development and Environs (Source: Google Maps, annotation by J.B. Barry & Partners)



Figure 2: Local Setting of Proposed Blackwood Square Development with the Northwood area (Source: Google Maps, annotation by J.B. Barry & Partners)

2.2 Land Use & Local Road Network

The existing site is a part greenfield and part temporary car park site. The temporary car park is often used for construction workers car parking from surrounding developments. Vehicular access is currently provided from the Affidea Northwood road through the existing Gulliver's Retail Park itself off Northwood Avenue. The site is situated on the eastern side of the Northwood area. The Site is bounded by Gulliver's Retail Park to the West, Cedarview residential development to the East and Swift Square office development to the East.

The land uses surrounding the development site are a mix of commercial and residential (comprising both individual dwellings and larger residential apartment blocks), all of which benefit from access to / from Northwood Avenue.

The Site is zoned "ME – Metro Economic Corridor" within the Development Plan with an objective description of:

"Facilitate opportunities for high-density mixed-use employment generating activity and commercial development, and support the provision of an appropriate quantum of residential development within the Metro Economic Corridor"

The Zoning objective vision states;

"Provide for an area of compact, high intensity/density, employment generating activity with associated commercial and residential development which focuses on the Metro within a setting of exemplary urban design, public realm streets and places, which are permeable, secure and within a high-quality green landscape. Landmark buildings will provide strong quality architectural features, which respect and enhance the character of the area into which they sit. The designated areas will form sustainable districts which possess a high degree of connectivity and accessibility and will be developed in a phased manner subject to the necessary provision of social and physical infrastructure."

2.3 Public Transport

An audit of the existing and proposed facilities and nearby transport was undertaken for the development site in the course of developing this Traffic and Transport Assessment. The Blackwood Residential Development has the M50 road in close proximity, allowing easy access to and from the area by car from outer areas. The site of the proposed development is located close to number of bus routes and a number of emerging transport developments. The audit considered the quality and availability of the existing facilities and public transport services. The audit found that subject site and surrounding lands are currently very well serviced by public transport.

The existing public transport facilities and emerging transport developments in the area surrounding the Northwood area are detailed following.

2.3.1 Pedestrian and Cycle Infrastructure

As a modern development, the pedestrian and cycle facilities within the Northwood area are of a good quality. All pedestrian routes leading to / from the development benefit from the provision of street lighting in addition to good quality pedestrian footways. There are numerous pedestrian crossing facilities available along Northwood Avenue just south of the development. Additionally, off road cycle tracks are provided throughout the Northwood area and on the external road network. Ballymun Road has an off-road cycle track while Swords Road has an On-Road cycle track. Figure 3 below illustrates the existing cycle network in the area and Figure 4 illustrates the proposed improvements from the NTA's "Greater Dublin Area Cycle Network Plan". The off-road cycle lane along Northwood Avenue branches out at numerous locations along the route providing additional cycle facilities throughout the Northwood area. For the Proposed Cycle

Network (Figure 4), it is noted that much of the alignment for the Santry River Greenway is already in place throughout the Northwood area.

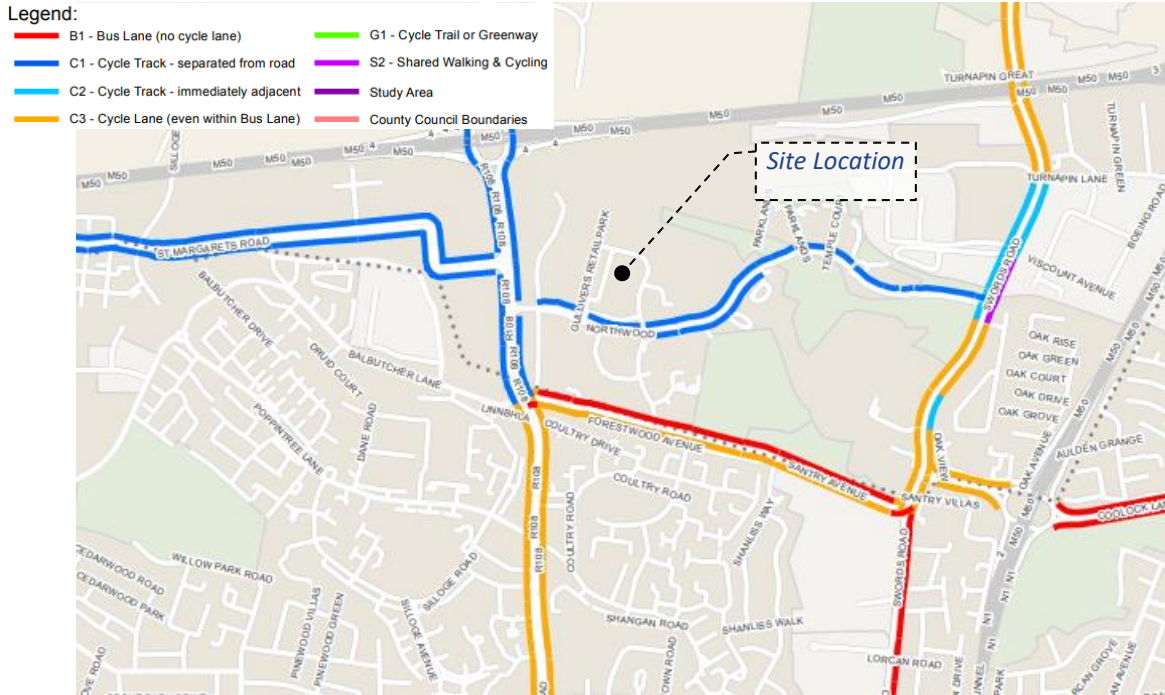


Figure 3: Existing Cycle Facilities - Greater Dublin Area Cycle Network Plan

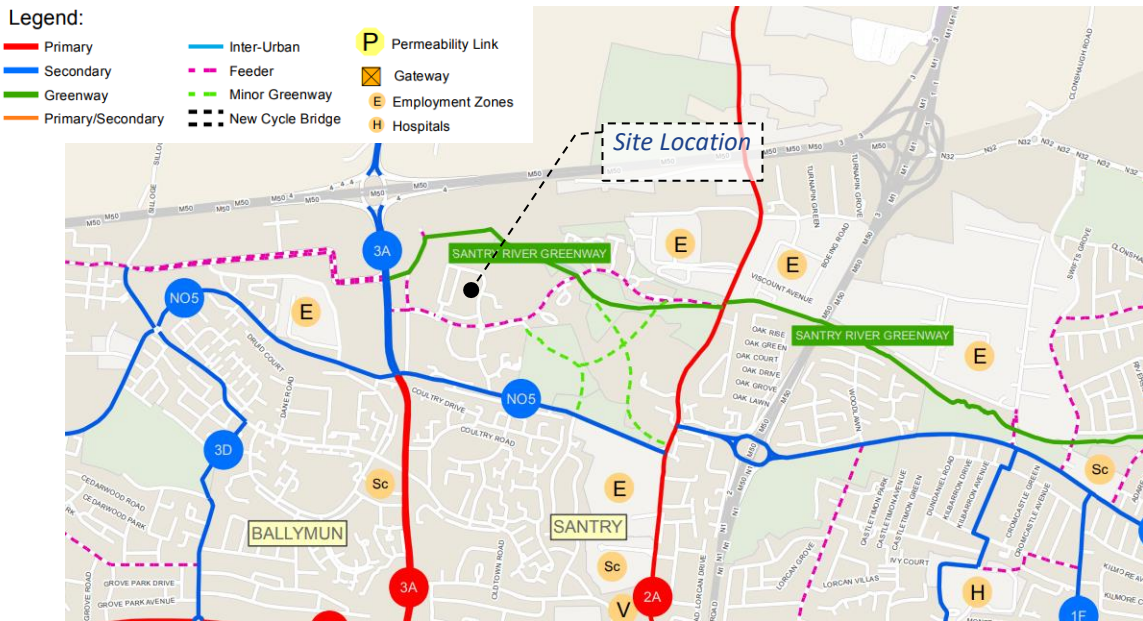


Figure 4: Proposed Cycle Facilities - Greater Dublin Area Cycle Network Plan

2.3.2 Existing Public Transport

The Site is ideally situated to benefit from a comprehensive range of Dublin Bus and Transport for Ireland (TFI) Go Ahead bus connections. Furthermore, the range and proximity of a number of emerging public transport interchanges further enhances the sustainability characteristics of the Site.

Dublin Bus and TFI operate numerous routes along Swords Road and Ballymun Road and Santry Avenue. These Dublin Bus operated bus services operate on a daily basis and offer relatively frequent schedules as summarised in **Table 1** below.

Table 1: Dublin Bus Service Frequency - No. of Services (www.dublinbus.ie)

Route No.	Route	Mon - Fri	Sat	Sun	
16	Dublin Airport to Ballinteer	87	81	63	Swords Road
27b	Eden Quay to Harristown	53	51	31	Swords Road
33 (TFI)	Lower Abbey St. to Balbriggan	21	14	12	Swords Road
41*	Lwr. Abbey St. to Swords Manor	50	44	29	Swords Road
41b	Lwr. Abbey St. to Rolestown	5	4	3	Swords Road
41c*	Lwr. Abbey St. to Swords Manor	45	41	28	Swords Road
4	Harristown to Monkstown Avenue	80	64	47	R108
155	Ikea (Ballymun) to Bray Rail Station	5	53	47	R108
13	Harristown to Grange Castle	75	63	46	R108
42d	DCU to Portmarnock	1	No service	No service	R108
17a (TFI)	Kilbarrack to Blanchardstown	57	52	42	Santry Avenue (R104)

2.3.3 Emerging Transport Developments

Bus Connects

Bus Connects proposes 16 No. Core Bus Corridors extending radially from Dublin City Centre to the surrounding suburbs. Dublin Bus Connects proposes to introduce numerous new bus routes in close proximity to the development. **Figure 5** taken from the latest Bus Connects proposal illustrates proposed new routes in the vicinity of the proposed development such as the “E Bus Route”, which is the Ballymun to City Centre Core Bus Corridor. The “E Spine” Core Bus Corridor will operate every 5 minutes or better. It would travel from the city centre along the R108/Ballymun Road and would split into branches, the E1 continuing north along the R108 (just west of the development) and the E2 would travel west towards Charlestown Shopping Centre. Each branch would operate every 10 minutes. The E1 would also operate to Bray and the E2 to Dun Laoghaire. Additionally, the A2 branch of the A Core Bus Corridor from Airport to Tallaght, A4 Branch of the A Core Bus Corridor between Swords to Rathfarnham and N8 Charlestown Shopping Centre to Howth Junction will all be frequent services.



Figure 5: Proposed Bus Connects Routes beside Northwood (Source www.busconnects.ie)

MetroLink

A Metro Link stop is currently proposed at Northwood in close proximity to the proposed development. Metro Link is the proposed high-capacity, high-frequency rail line running from Swords to Charlemont, linking Dublin Airport, Irish Rail, DART, Dublin Bus and Luas services, creating a fully integrated public transport service in the Greater Dublin Area. The proposed Northwood Metro Link stop will likely be located west of the Blackwood Square Development at the junction of the Ballymun Road (R108)/Northwood Avenue. The exact location is yet to be confirmed. The development will also include a new pedestrian walkway through Gulliver’s Retail Park, providing direct access to the MetroLink stop. The new pedestrian walkway will require removing c.44 car parking spaces in the retail park. It is noted that the retail park is rarely over 50% occupancy due to the excessive number of parking spaces. See **Figure 6** and **Figure 7** for the route map and a draft artists impression of the Northwood stop by the Ballymun Road.



Figure 6: MetroLink Emerging Preferred Route and Northwood Stop (Source www.metrolink.ie)



Figure 7: Artists impression of proposed Northwood Stop on Ballymun Rd (R108) (Source www.metrolink.ie)

SECTION 3: BASE YEAR 2019 – TRAFFIC VOLUMES & CAPACITY

3.1 Traffic Survey

In order to assess the operation of the proposed road network and its future capacity, a traffic model of the existing local road network and proposed links was created. Firstly, a vehicle turning movement survey was carried out at seven junctions near the subject site (See Figure 8);

- Site 1- Junction 1) Ballymun Road (R108) / St. Margaret's Rd;
- Site 2- Junction 2) Ballymun Road (R108) / Northwood Avenue;
- Site 3- Junction 3) Northwood Avenue / Old Ballymun Road;
- Site 4- Junction 4) Northwood Avenue / Affidea Northwood;
- Site 5- Junction 5) Northwood Ave / Northwood Road;
- Site 6- Junction 6) Santry Ave / Northwood Road;
- Site 7- Junction 7) Northwood Ave / Swords Road (R138):

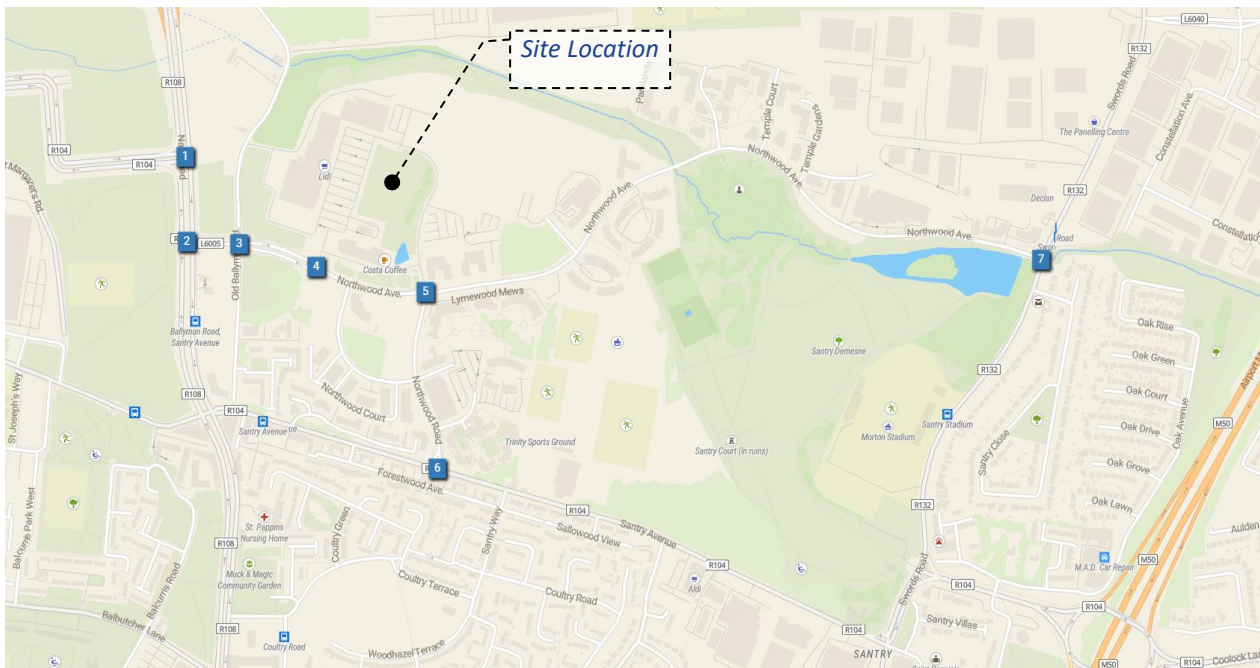


Figure 8 - Traffic Survey Locations

The counts captured all turning movements at these junctions. The vehicle turning movement surveys were undertaken on Tuesday 12th February 2019. The counts were carried out over the 12-hour period 07:00 hours to 19:00 hours including both the morning and evening peak periods.

The morning peak hour was identified as 08:00-09:00 for all junctions. The evening peak hour was identified as 17:00-18:00 for all junctions. Data was collected in 15-minute intervals. A full transcription of the turning movement survey is included in Appendix 1 herein.

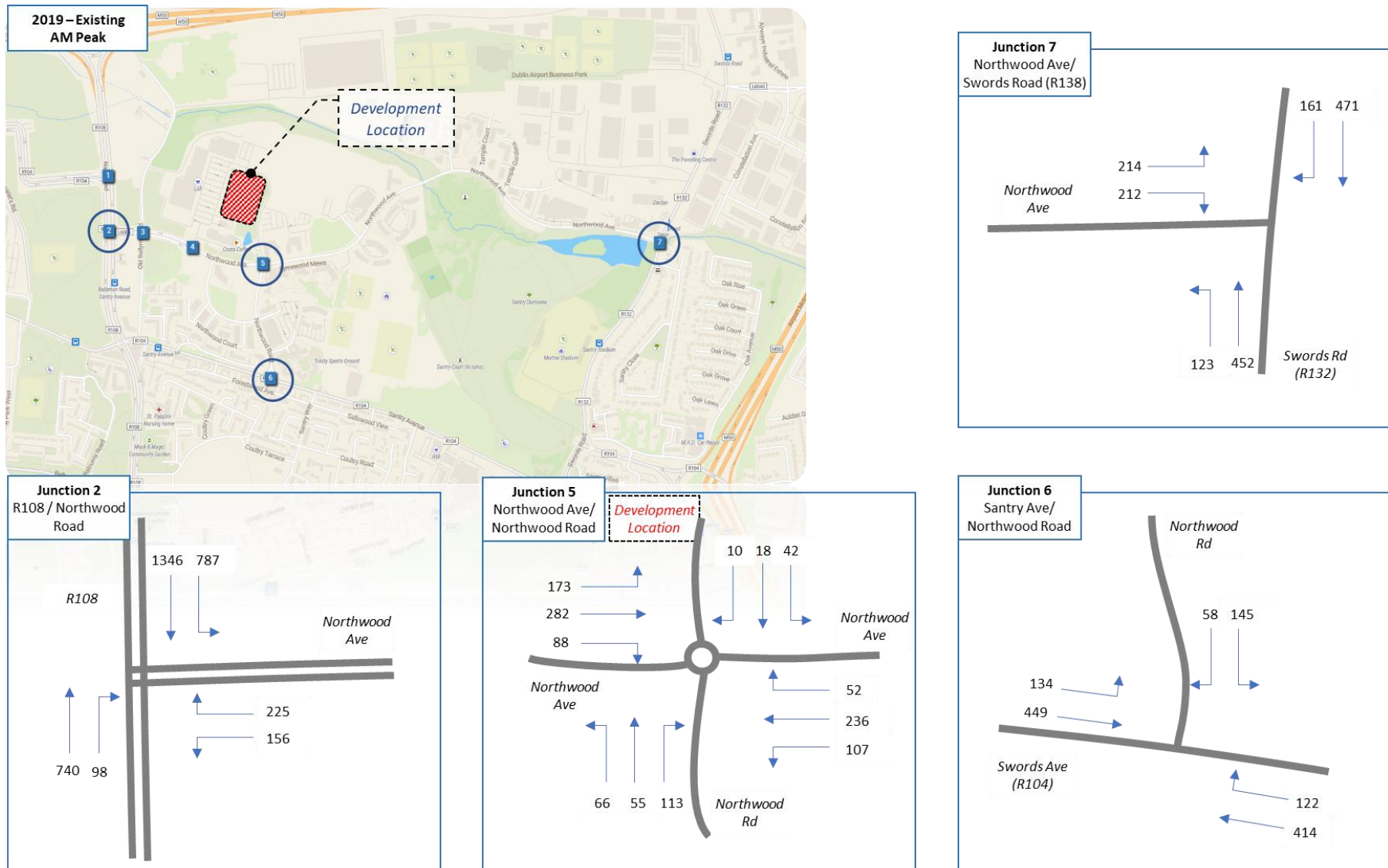
The evening peak hour of 17:00 to 18:00 hours was observed to be marginally more intense than the morning peak hour. However, in order to carry out a robust traffic analysis of the surrounding road network, the traffic modelling exercise following herein will be based on traffic flows recorded for both the weekday morning and evening peak hours for each junction.

A summary of the 2019 vehicle turning movement surveys for the morning and evening peak hour periods is shown in Figure 9 and Figure 10 below.

In order to determine the effect of the development on the adjoining road network, the estimated trip generation (as detailed in the next section below), was applied to the traffic counts at each junction. It was assumed the trips generated by the development will mirror the existing trip distribution.

It was determined that Junction 2) Ballymun Road (R108) / Northwood Avenue, Junction 6) Santry Ave / Northwood Road and Junction 7) Northwood Ave / Swords Road (R138) were the key junctions to be modelled, as they provide access to/from the public road network. Junction 5) Northwood Ave / Northwood Road, whilst fully contained within the private area of Northwood, would experience the largest increase from trips generated by the development and therefore would also be modelled.

As part of the OSCADY traffic models, existing signal timings were recorded for each of the signalised junctions. The existing traffic signals were vehicle actuated on each arm and as a result, signal and cycle timings differed depending on the traffic flows. For the purpose of the OSCADY models, numerous signal timings were recorded, and an average signal time was utilised for the model. This was carried out for the morning and evening peak hour. Additionally, as part of the traffic analysis, optimised signal and cycle timings were modelled. The optimised timings from OSCADY were largely similar to the recorded results, demonstrating that the existing vehicle actuated timings are operating efficiently as possible.



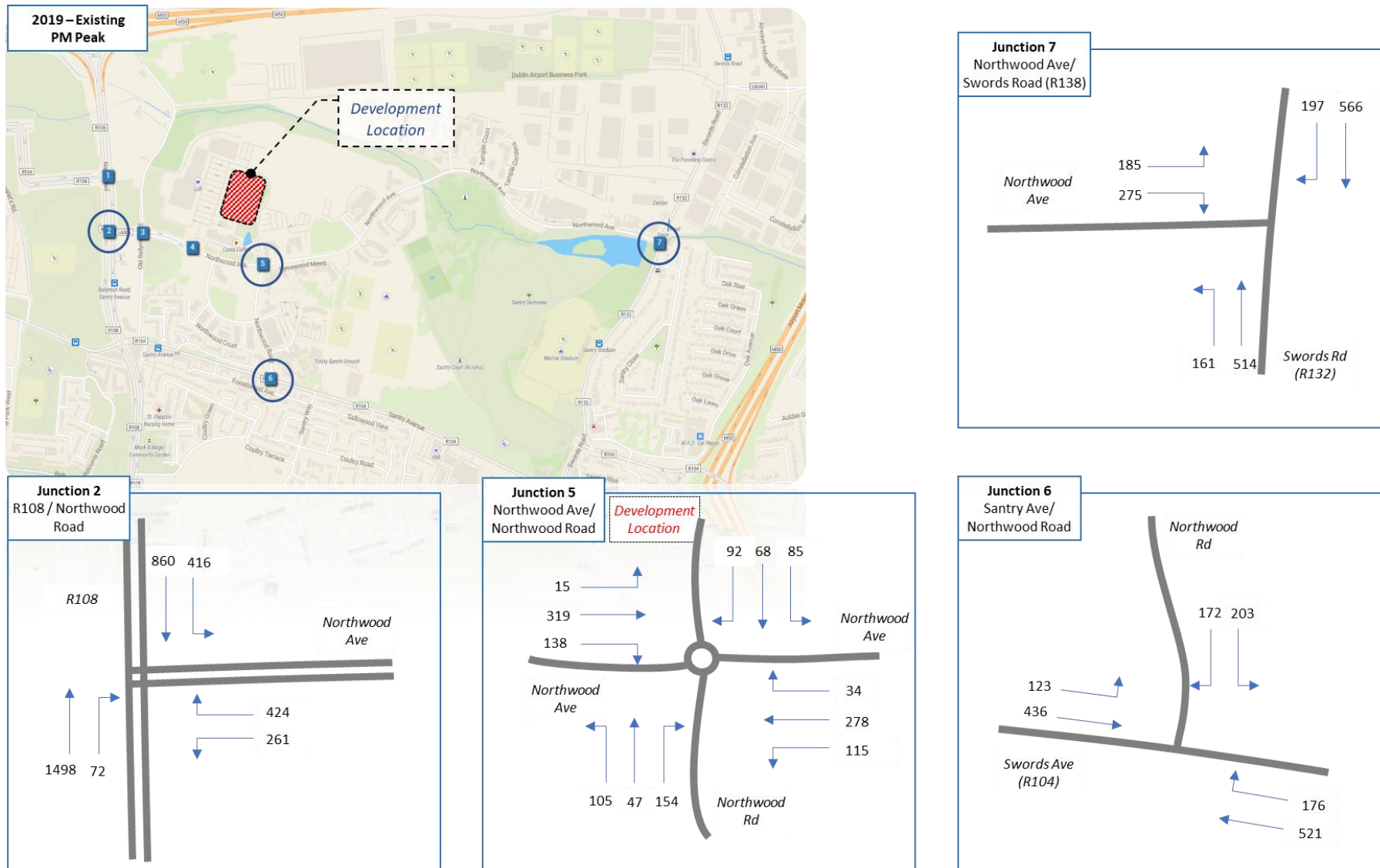


Figure 10: Vehicle turning movement- 2019 Existing Evening Peak Hour (17:00 – 18:00)

3.2 Junction Capacity Assessment for Base Year 2019

A traffic capacity assessment of the four key junctions in the vicinity the subject site was undertaken utilising the surveyed results shown in **Table 2** and **Table 3** above and TRL's OSCADY (for junctions 2, 6 and 7) & ARCADY (for junction 5) traffic modelling software.

A summary of the results of the analysis of Junction 2) Ballymun Road (R108) / Northwood Avenue, Junction 5) Northwood Ave / Northwood Road, Junction 6) Santry Ave / Northwood Road and Junction 7) Northwood Ave / Swords Road (R138) for the morning and evening peak hours is shown in **Table 4** and **Table 5** following.

**Table 2: Junction 2) Ballymun Road (R108) / Northwood Avenue
2019 Morning and Evening Peak Hour**

Approach Arm	Max. RFC		Max. Queue (Vehicles)		Average Delay (Seconds)	
	AM	PM	AM	PM	AM	PM
Ballymun Road (R108) North	0.841	0.510	14	12	26	30
Northwood Avenue	0.904	0.934	10	23	86	97
Ballymun Road (R108) South	0.316	0.642	4	15	8	18

**Table 3: Junction 5) Northwood Ave / Northwood Road
2019 Morning and Evening Peak Hour**

Approach Arm	Max. RFC		Max. Queue (Vehicles)		Average Delay (Seconds)	
	AM	PM	AM	PM	AM	PM
Affidea Northwood	0.047	0.177	1	1	2	2
Northwood Avenue East	0.244	0.286	1	1	2	2
Northwood Road	0.139	0.191	1	1	2	2
Northwood Avenue West	0.330	0.289	1	1	3	3

**Table 4: Junction 6) Santry Ave / Northwood Road
2019 Morning and Evening Peak Hour**

Approach Arm	Max. RFC		Max. Queue (Vehicles)		Average Delay (Seconds)	
	AM	PM	AM	PM	AM	PM
Santry Ave West	0.619	0.614	9	9	22	23
Northwood Road	0.357	0.609	4	7	25	28
Santry Ave East	0.589	0.788	5	9	11	18

**Table 5: Junction 7) Northwood Ave / Swords Road (R138)
2019 Morning and Evening Peak Hour**

Approach Arm	Max. RFC		Max. Queue (Vehicles)		Average Delay (Seconds)	
	AM	PM	AM	PM	AM	PM
Swords Road South	0.670	0.791	8	14	28	43
Northwood Avenue	0.660	0.840	9	15	33	56
Swords Road North	0.825	0.939	10	16	23	34

The normal design threshold for the ratio of flow to capacity (RFC) is 0.9 for a signalised junction and 0.85 for a roundabout junction.

Table2 and **Table5** demonstrate that Junction 2) Ballymun Road (R108) / Northwood Avenue currently operates just over the normal design threshold during the morning and evening peak hours considered. This is evident at the Northwood Avenue arm with queues and delays for motorists forming. Junction 7) Northwood Ave / Swords Road (R138) will exceed the normal design threshold on the Swords Road arm during the evening peak hour. This analysis concurs with observations made on site, as minor queuing was apparent at each of the junctions. Junction 5) Northwood Ave / Northwood Road and Junction 6) Santry Ave / Northwood Road currently operate efficiently below the design threshold. Furthermore, this analysis concurs with the “max” queue length survey taken at the same time during the traffic counts on Tuesday 12th February 2019. The max queue lengths are largely similar to the modelled queue lengths across the board.

At peak traffic times, such as the evening peak hour, traffic occasionally queues from the Ballymun Road signalised junction along Northwood Avenue in both traffic lanes through the Old Ballymun Road junction and into the Northwood area. Traffic is not observed to keep the junction clear during such instances and blocks entry and exit movements into and out of Old Ballymun Road.

SECTION 4: CHARACTERISTICS OF THE PROPOSED SCHEME & ACCESS

4.1 Overview

The proposed development will consist of 331 apartments in four separate blocks, with retail units, a gym area and a crèche at ground floor level over basement car parking, and all associated site works including roads, footpaths, landscaping, site services, SuDS measures and sundry related works. A full project description is contained in Section 1 and 2 of this TTA.

4.2 Site Access

The Site will benefit from one vehicle access which will be provided on Affidea Northwood Road, directly north of the site itself. This access forms part of an existing mini roundabout on Affidea Northwood Road, with arms to Gulliver's Retail Park (west arm), the Cedarview development (north arm), the Blackwood Square development (south arm) and Affidea Northwood Road itself. See Figure 11 for a sketch illustrating the Affidea Northwood Road roundabout.

Affidea Northwood Road connects to the existing roundabout junction on Northwood Avenue. This will form part of the main access to the development, as all residential traffic will likely come through this Northwood Avenue roundabout. Whilst it will be possible to access the development through the Gulliver's Retail Park car park, the route contains additional turning movements and pedestrian crossing points and therefore it is unlikely that any residential traffic will take this route.



Figure 11: Proposed Access Arrangement Affidea Road (Source: Google Maps)

4.3 Road Safety Assessment

The Road Safety Authority (RSA) database of road collision information was interrogated to establish if the surrounding road network in the vicinity of the proposed development access holds records relating to historical collision occurrence (Figure 12 below). Collisions from 2005 to 2015 only are available.

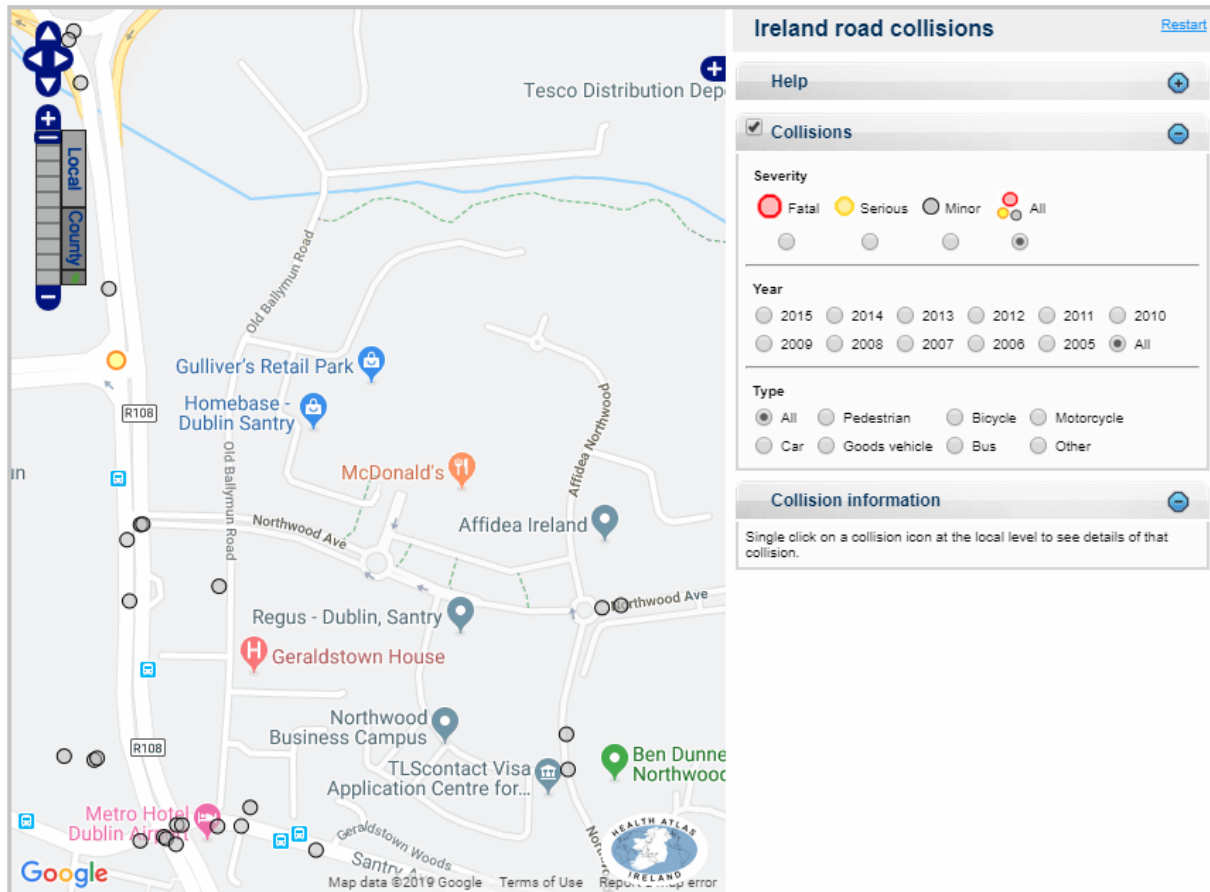


Figure 12: Historic Collisions 2005 - 2015 (Source RSA Database)

This exercise revealed that there have been two minor collisions near the site on Northwood Avenue, both recorded in 2013. One was a single vehicle collision, the other a pedestrian/vehicle collision. In addition, there has been two minor collisions on Northwood Road, one involving a bicycle in 2014 and one involving two vehicles in 2008. A number of minor collisions have also taken place outside the Northwood area on the Ballymun Road (R108) at the junction with Northwood Avenue and the junction with Santry Avenue. Due to the isolated nature and low frequency of these collisions a pattern of collisions is not identifiable.

SECTION 5: TRIP GENERATION AND ASSIGNMENT

5.1 Trip Generation

The Trip Rate Information Computer System (TRICS) database was interrogated to derive the potential development trip generation rates. Utilising data supplied by the TRICS database, **Table 6** details the estimated trip generation for the proposed residential units, retail area, childcare facility and gym development during the morning and evening peak hours being considered for this study. The TRICS morning and evening peak hours were 08:00 to 09:00 and 17:00 to 18:00 respectively. The trips generated during these times were applied to the morning and evening peak hours for the road network.

As the residential element of the Blackwood Square development includes 1, 2 and 3 bedroom apartments, the trip rates were calculated “per bedroom” in order to produce a more robust result. The full TRICS output files are contained in Appendix 2.

In reality, the trips generated by the retail, childcare facility and gym area will likely come from within the Northwood development and most likely form part of the residential trips (ie. the people using the retail area will live within the Blackwood Square development or nearby Northwood development, rather than the surrounding Ballymun / Santry area). However, in order to produce a robust, conservative and a worst-case scenario, it will be assumed that all trips generated by the development will be generated from outside the Northwood area.

Table 6 - Trip Generation

	Time	Factor	TRICS Arrival Rate	TRICS Departure Rate	Hourly Trips	
					Trips In	Trips Out
Residential Development 331 Units	Morning Peak Hour	689 Bedrooms	0.028 <i>(per bedroom)</i>	0.106 <i>(per bedroom)</i>	19	73
	Evening Peak Hour		0.098 <i>(per bedroom)</i>	0.041 <i>(per bedroom)</i>	67	28
Retail	Morning Peak Hour	1,006 m ²	2.899 <i>(per 100m²)</i>	2.357 <i>(per 100m²)</i>	29	24
	Evening Peak Hour		4.247 <i>(per 100m²)</i>	4.789 <i>(per 100m²)</i>	43	48
Childcare facility	Morning Peak Hour	321 m ²	4.235 <i>(per 100m²)</i>	3.286 <i>(per 100m²)</i>	14	11
	Evening Peak Hour		2.970 <i>(per 100m²)</i>	3.547 <i>(per 100m²)</i>	10	11
Gym	Morning Peak Hour	126 m ²	0.841 <i>(per 100m²)</i>	0.654 <i>(per 100m²)</i>	1	1
	Evening Peak Hour		1.684 <i>(per 100m²)</i>	0.981 <i>(per 100m²)</i>	2	1
TOTAL	Morning Peak Hour				63	109
	Evening Peak Hour				122	88

5.2 Modal Choice

When estimating trip generation for a residential development using TRICS the trip rate for car drivers generally accounts for a 65% modal split of total trips. In order to produce a robust and conservative assessment of the traffic impact of the proposed Blackwood Square development, this study will continue to utilise the modal split of 65% for car trips. In reality, this modal split will be smaller as a number of residents will choose to take the Dublin Bus service in the vicinity of the development and MetroLink in the future. Additionally, the parking strategy has been development to encourage less of a reliance on private cars and a greater focus on sustainable transport such as cycling.

5.3 Trip Distribution and Assessment Years

It was assumed for the purposes of this study that the future development traffic will mirror existing travel flows when exiting and entering the development. The existing traffic from the entire Northwood development was analysed in the morning and evening peak hours. Currently during the morning peak, 42% of vehicles departing the Northwood area travel east towards the Swords Road, 20% travel south towards Santry Ave, while the remaining 38% travel west towards the Ballymun Road. During the evening peak hour 30% travel east towards the Swords Road, 25% travel south towards Santry Avenue and 45% travel west towards the Ballymun Road. The future development traffic distribution at the surrounding junctions will mirror existing traffic patterns i.e. development generated flows will be split through the junctions proportionally to existing flows.

Assuming planning permission is granted for the development in 2019, and allowing for a 2 to 3 year construction period, it is estimated that the proposed development will be fully operational by 2021/2022. For the purpose of this study, the end of 2021 is assumed as the Year of Opening. Therefore, traffic analysis associated with this study will focus on the following future development operational scenarios:

- Residential Development Year of Opening – 2021
- 15 Year Design Horizon – 2036

The projected 2021 and 2036 design year traffic flows have been calculated by factoring up the 2019 recorded traffic flows in accordance with the TII Publications Project Appraisal Guidelines for National Roads document 'Unit 5.3 Travel Demand Projections, Table 5.3.2: Link-Based Growth Rates: Annual Growth Factors. The medium growth rate factors have been utilised. The TII traffic growth rates will account for any increase in traffic as a result of other developments in the area such as Bridgefield, Cedarview and other potential developments in the future. Consequently, all impacts assessed are inherently cumulative impacts.

Figure 13 and Figure 14 illustrate the 2021 Year of Opening for the “without” and **“with”** development scenarios for morning and evening peaks. Figure 15 and Figure 16 illustrate the 2036 Design Year Horizon for the “without” and **“with”** development scenarios for morning and evening peaks.



Figure 13: Vehicle turning movement 2021 Opening Year - Morning Peak Hour (08:00 – 09:00)



Figure 14: Vehicle turning movement 2021 Opening Year - Evening Peak Hour (17:00 – 18:00)

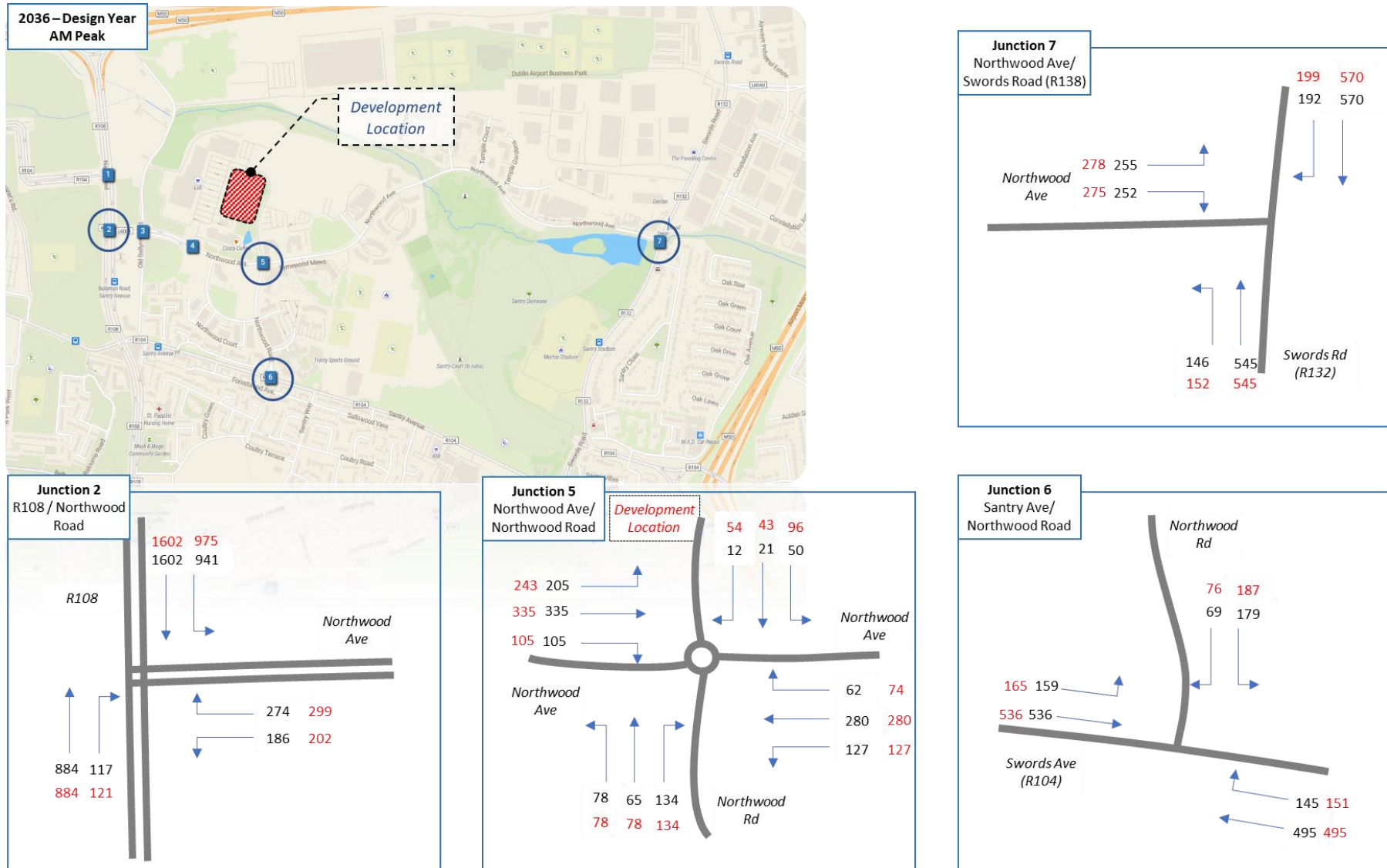


Figure 15: Vehicle turning movement 2036 Design Year - Morning Peak Hour (08:00 – 09:00)

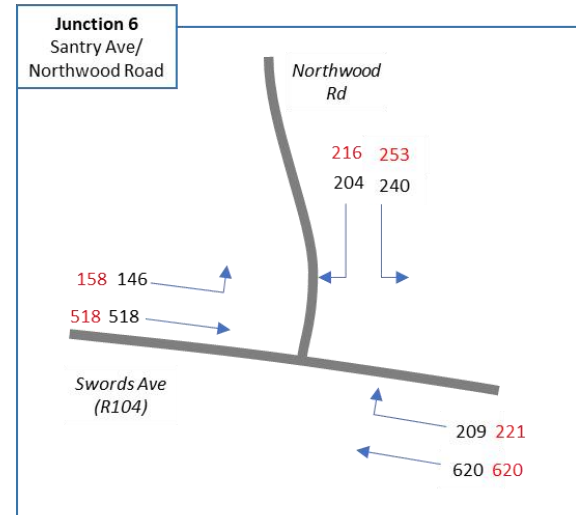
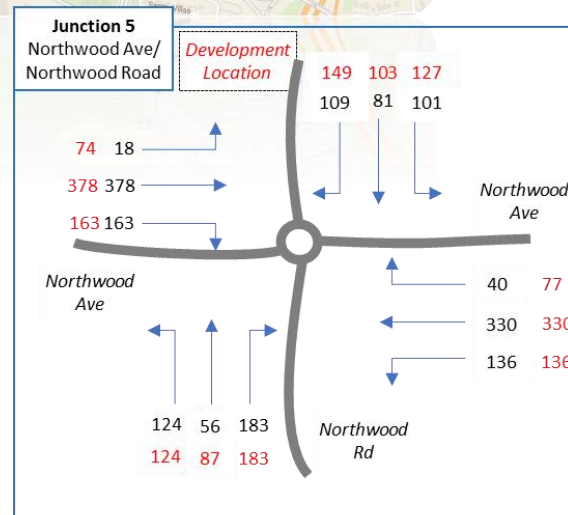
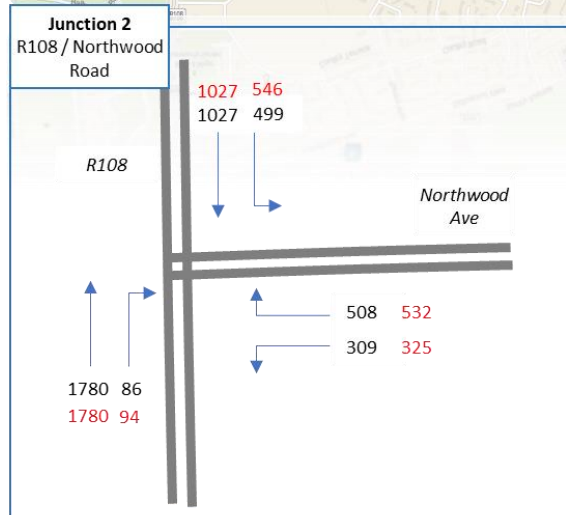
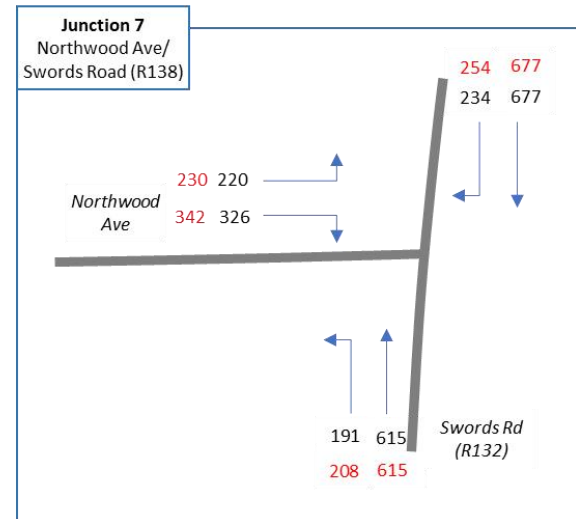
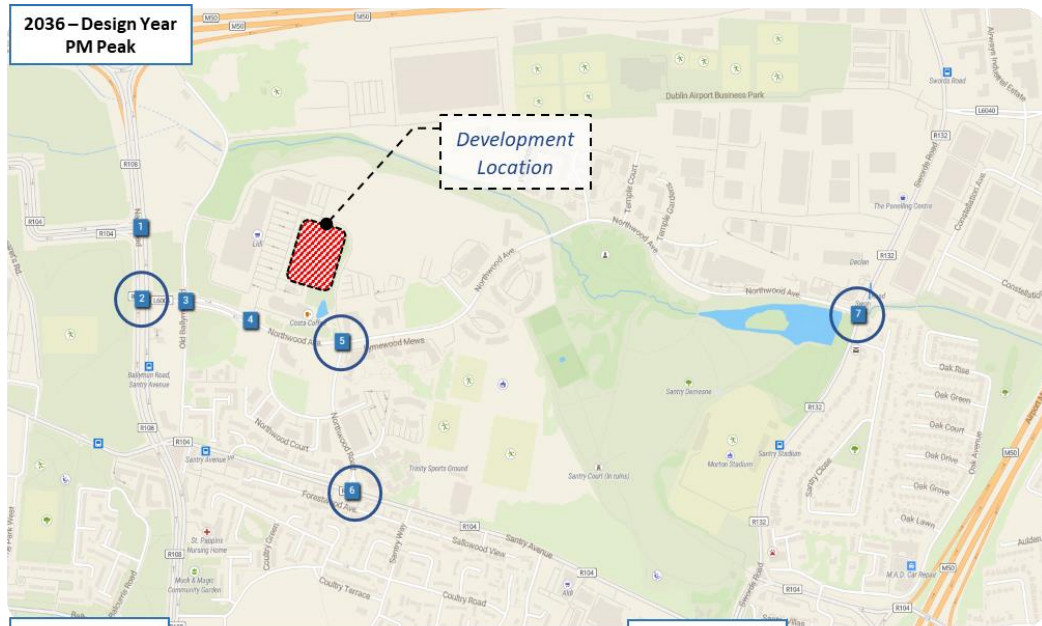


Figure 16: Vehicle turning movement 2036 Design Year - Evening Peak Hour (17:00 – 18:00)

SECTION 6: TRAFFIC IMPACT

6.1 Background

In order to assess the future traffic impact of the proposed development, capacity assessments were undertaken using TRL's OSCADY (for junctions 2, 6 and 7) & ARCADY (for junction 5) software on the following junctions;

- Site 2- Junction 2) Ballymun Road (R108) / Northwood Avenue;
- Site 5- Junction 5) Northwood Ave / Northwood Road;
- Site 6- Junction 6) Santry Ave / Northwood Road;
- Site 7- Junction 7) Northwood Ave / Swords Road (R138)

The junctions were modelled for the 2021 year of Opening and 2036 the 15 Year Design Horizon for the morning and evening peak hour periods using the flow diagrams shown in Figure 13 to Figure 16 in the previous section herein.

To demonstrate the direct traffic impact associated with the proposed development on the key junction being considered, the traffic modelling exercise was carried out for the "Without" development and "with" development scenarios. A sample traffic modelling output file is included in this report in Appendix 3.

6.2 Operational Phase 2021 Traffic Impact

A traffic capacity assessment of the four junctions in the vicinity the subject site was undertaken utilising the surveyed results shown in **Figure 13** and **Figure 14** above and TRL's OSCADY (for junctions 2, 6 and 7) & ARCADY (for junction 5) traffic modelling software.

A summary of the results of the analysis of Junction 2) Ballymun Road (R108) / Northwood Avenue, Junction 5) Northwood Ave / Northwood Road, Junction 6) Santry Ave / Northwood Road and Junction 7) Northwood Ave / Swords Road (R138) both "without" and "with" the development for the morning and evening peak hours is shown in **Table 7** to **Table 10** following.

Table 7: Junction 2) Ballymun Road (R108) / Northwood Avenue
2021 Morning and Evening Peak Hour

Approach Arm	Max. RFC		Max. Queue (Vehicles)		Average Delay (Seconds)	
	AM	PM	AM	PM	AM	PM
Ballymun Road (R108) North	0.865 0.901	0.525 0.582	15 16	12 13	28 31	31 32
Northwood Avenue	0.932 1.032	0.960 1.013	12 19	25 32	94 143	104 125
Ballymun Road (R108) South	0.335 0.348	0.660 0.660	4 4	15 15	8 9	19 20

The normal design threshold for the ratio of flow to capacity (RFC) is 0.9 for a signalised junction. **Table 7** demonstrates that at Junction 2) Ballymun Road (R108) / Northwood Avenue, the Ballymun Road arm will

operate just within the normal design threshold during the morning and evening peak hours considered. However, the Northwood Avenue arm, will exceed the design threshold with queues and delays for motorists evident. This is the case both “without” and “with” the development scenarios. The analysis indicates that the development will have an imperceptible impact on the Junction 2) Ballymun Road (R108) / Northwood Avenue junction. During consultation with FCC, it is noted that Fingal plan to upgrade Junction 3) Northwood Avenue / Old Ballymun Road to incorporate SCATS. Upgrading this junction to SCATS will allow the junction to control the traffic arriving from Northwood to the Ballymun Road. The junctions will better calculate and adapt the timing of traffic signals in the network allowing the junction to operate efficiently, creating an overall positive impact.

**Table 8: Junction 5) Northwood Ave / Northwood Road
2021 Morning and Evening Peak Hour**

Approach Arm	Max. RFC		Max. Queue (Vehicles)		Average Delay (Seconds)	
	AM	PM	AM	PM	AM	PM
Affidea Northwood	0.048 0.123	0.183 0.247	1 1	1 1	2 2	2 3
Northwood Avenue East	0.250 0.267	0.295 0.329	1 1	1 1	2 2	3 3
Northwood Road	0.143 0.155	0.197 0.225	1 1	1 1	2 2	2 2
Northwood Avenue West	0.340 0.367	0.297 0.342	1 1	1 1	3 3	2 3

The normal design threshold for the ratio of flow to capacity (RFC) is 0.85 for a roundabout junction. **Table 8** demonstrates that the Junction 5) Northwood Ave / Northwood Road roundabout will operate within the normal design threshold during the morning and evening peak hours considered. This is the case in both “without” and “with” the development scenarios. The analysis indicates that despite a large increase in traffic on the Affidea Northwood arm, as the junction is considerably within capacity, the development will have an insignificant impact on the operation of the roundabout.

**Table 9: Junction 6) Santry Ave / Northwood Road
2021 Morning and Evening Peak Hour**

Approach Arm	Max. RFC		Max. Queue (Vehicles)		Average Delay (Seconds)	
	AM	PM	AM	PM	AM	PM
Santry Ave West	0.636 0.642	0.630 0.646	9 9	9 10	23 23	24 24
Northwood Road	0.367 0.404	0.626 0.666	4 4	7 8	25 26	29 29
Santry Ave East	0.609 0.620	0.815 0.835	5 6	9 10	12 12	19 20

The normal design threshold for the ratio of flow to capacity (RFC) is 0.9 for a signalised junction. **Table 9** demonstrates that Junction 6) Santry Ave / Northwood Road will operate within the normal design threshold during the morning and evening peak hours considered. Very little queues and delays are forming in both the “without” and “with” development scenarios. The analysis indicates that the development will have an insignificant impact on the Junction 6) Santry Ave junction.

**Table 10: Junction 7) Northwood Ave / Swords Road (R138)
2021 Morning and Evening Peak Hour**

Approach Arm	Max. RFC		Max. Queue (Vehicles)		Average Delay (Seconds)	
	AM	PM	AM	PM	AM	PM
Swords Road South	0.689 0.689	0.812 0.812	9 9	15 15	28 28	44 44
Northwood Avenue	0.678 0.750	0.862 0.911	10 11	16 18	34 36	58 64
Swords Road North	0.859 0.876	0.975 1.020	11 11	21 31	25 26	48 59

The normal design threshold for the ratio of flow to capacity (RFC) is 0.9 for a signalised junction. **Table 10** demonstrates that Junction 7) Northwood Ave / Swords Road (R138) will operate within the normal design threshold during the morning peak hour but exceed the normal design threshold during the evening peak hour. This is the case both “without” and “with” the development scenarios. At this stage, once a junction is at capacity any slight increase, whether it is background traffic growth or new trip generation, will have a noticeable increase in queues/delays.

The analysis indicates that the junction will not operate efficiently in either the “without” and “with” the development scenarios. Any future traffic growth, irrespective of the subject development, will therefore result in an impact to the operation of the junction. However, the Blackwood Square development will have

little effect on the junction. Additionally, the expected improvements in the public transport services in the surrounding area will have a positive impact on junction capacity.

A sample of the traffic modelling output files are included in this report in Appendix 3.

6.3 Operational Phase 2036 Traffic Impact

A traffic capacity assessment of the four key junctions in the vicinity the subject site was undertaken utilising the surveyed results shown in Figures 15 and 16 above and TRL's OSCADY (for junctions 2, 6 and 7) & ARCADY (for junction 5) traffic modelling software.

A summary of the results of the analysis of Junction 2) Ballymun Road (R108) / Northwood Avenue, Junction 5) Northwood Ave / Northwood Road, Junction 6) Santry Ave / Northwood Road and Junction 7) Northwood Ave / Swords Road (R138) both "without" and "with" the development for the morning and evening peak hours is shown in **Table 11** to **Table 14** following.

**Table 11: Junction 2) Ballymun Road (R108) / Northwood Avenue
2036 Morning and Evening Peak Hour**

Approach Arm	Max. RFC		Max. Queue (Vehicles)		Average Delay (Seconds)	
	AM	PM	AM	PM	AM	PM
Ballymun Road (R108) North	0.989 1.025	0.612 0.670	30 40	14 16	51 69	34 35
Northwood Avenue	0.956 1.043	1.119 1.172	15 23	54 68	101 149	202 248
Ballymun Road (R108) South	0.482 0.499	0.763 0.763	5 5	18 18	13 9	22 22

The normal design threshold for the ratio of flow to capacity (RFC) is 0.9 for a signalised junction. **Table 11** demonstrates that Junction 2) Ballymun Road (R108) / Northwood Avenue will not operate within the normal design threshold during the morning and evening peak hours considered. This is the case in both "without" and "with" the development scenarios. The junction will also exceed the theoretical capacity of 1.0 for a signalised junction by 2036. At this stage, once a junction is at capacity any slight increase, whether it is background traffic growth or a new residential development, will have a noticeable increase in queues/delays. During consultation with FCC, it is noted that Fingal plan to upgrade Junction 3) Northwood Avenue / Old Ballymun Road to incorporate SCATS. Upgrading this junction to SCATS will allow the junction to control the traffic arriving from Northwood to the Ballymun Road. The junctions will better calculate and adapt the timing of traffic signals in the network allowing the junction to operate efficiently, creating an overall positive impact.

The analysis indicates that the junction will not operate efficiently in either the "without" and "with" the development scenarios. Any future background traffic growth, irrespective of the subject development, will therefore result in an impact to the operation of the junction. However, the Blackwood Square development will have an insignificant impact on the junction.

**Table 12: Junction 5) Northwood Ave / Northwood Road
2036 Morning and Evening Peak Hour**

Approach Arm	Max. RFC		Max. Queue (Vehicles)		Average Delay (Seconds)	
	AM	PM	AM	PM	AM	PM
Affidea Northwood	0.059 0.136	0.223 0.290	1 1	1 1	2 2	2 3
Northwood Avenue East	0.292 0.308	0.347 0.384	1 1	1 1	2 3	3 4
Northwood Road	0.169 0.182	0.235 0.265	1 1	1 1	2 2	2 3
Northwood Avenue West	0.400 0.429	0.349 0.396	1 1	1 1	3 3	3 3

The normal design threshold for the ratio of flow to capacity (RFC) is 0.85 for a roundabout junction. **Table 12** demonstrates that the Junction 5) Northwood Ave / Northwood Road roundabout will operate within the normal design threshold during the morning and evening peak hours considered. This is the case both “without” and “with” the development scenarios. The analysis indicates that despite a large increase in traffic on the Affidea Northwood arm, as the junction is considerably within capacity, the development will have an insignificant impact on the operation of the roundabout.

**Table 13: Junction 6) Santry Ave / Northwood Road
2036 Morning and Evening Peak Hour**

Approach Arm	Max. RFC		Max. Queue (Vehicles)		Average Delay (Seconds)	
	AM	PM	AM	PM	AM	PM
Santry Ave West	0.737 0.744	0.729 0.743	11 11	11 12	25 25	26 26
Northwood Road	0.436 0.462	0.722 0.762	4 5	9 9	26 26	31 33
Santry Ave East	0.732 0.743	0.979 1.001	7 7	20 25	15 15	40 47

The normal design threshold for the ratio of flow to capacity (RFC) is 0.9 for a signalised junction. **Table 13** demonstrates that Junction 6) Santry Ave / Northwood Road will operate within the normal design threshold during the morning and evening peak hours considered. Very little queues and delays are forming in both the “without” and “with” development scenarios. The analysis indicates that the development will have an insignificant impact on the Junction 6) Santry Ave junction.

Table 14: Junction 7) Northwood Ave / Swords Road (R138)
2036 Morning and Evening Peak Hour

Approach Arm	Max. RFC		Max. Queue (Vehicles)		Average Delay (Seconds)	
	AM	PM	AM	PM	AM	PM
Swords Road South	0.873 0.873	0.951 0.951	15 15	24 24	47 47	66 66
Northwood Avenue	0.881 0.961	1.122 1.176	16 22	55 70	55 70	190 238
Swords Road North	1.006 1.022	1.128 1.170	26 30	72 90	54 61	137 172

The normal design threshold for the ratio of flow to capacity (RFC) is 0.9 for a signalised junction. **Table 14** demonstrates that Junction 7) Northwood Ave / Swords Road (R138) will not operate within the normal design threshold during the morning and evening peak hours considered. The junction will also exceed the theoretical capacity of 1.0 for a signalised junction by 2036. This is the case in both “without” and **“with”** the development scenarios. At this stage, once a junction is at capacity any slight increase, whether it is background traffic growth or new trip generation, will have a noticeable increase in queues/delays.

The analysis indicates that the junction will not operate efficiently in either the “without” and **“with”** the development scenarios. Any future traffic growth, irrespective of the subject development, will therefore result in an impact to the operation of the junction. However, the Blackwood Square development will only have a minor impact on the junction. Additionally, the expected improvements in the public transport services in the surrounding area will have a positive impact on junction capacity.

6.4 Summary

The traffic analysis, traffic/queue counts, and on-site observations all demonstrated that Junction 2) Ballymun Road (R108) / Northwood Avenue and Junction 7) Northwood Ave / Swords Road (R138), are currently nearing capacity. In the future, the junctions will not operate efficiently in either the “without” and **“with”** the development scenarios. Any future traffic growth, irrespective of the subject development, will therefore result in an impact to the operation of the junction. The planned upgrade of the Old Ballymun Road junction to SCATS will help control traffic reducing delays. It is noted that the 2036 analysis does not include the likely improvements in the public transport services (MetroLink and BusConnects). Furthermore, in order to produce a robust, conservative analysis, a worst-case scenario was assumed as noted in Section 5.1 Trip Generation.

This further emphasises the need for an improved public transport network as outlined in Section 2.3, such as MetroLink and BusConnects, regardless of the Blackwood Square development. Additionally, further investments are needed in cycling infrastructure not just in Northwood, but the Greater Dublin Area. This will encourage a greater number of Northwood residents to leave the car at home and choose sustainable transport modes.

6.5 Cumulative Impacts

The TII traffic growth rates will account for any increase in traffic as a result of other developments in the area such as Bridgefield, Cedarview and other potential developments in the future. Consequently, all impacts assessed are inherently cumulative impacts.

6.6 Traffic Impacts During Construction

All construction activities will be governed by a construction Traffic Management Plan (TMP) the details of which will be agreed with FCC's Roads Department prior to the commencement of the Construction Phase. The principal objective of the TMP is to ensure that the impacts of all building activities generated during the Construction Phase upon both the public (off-site) and internal (on site) workers environments, are fully considered and proactively managed / programmed respecting key stakeholders' requirements.

During the construction works there will be additional HGV movements to/from the Site. Traffic will be generated by the disposal of surplus subsoil from the Site, deliveries of construction materials and equipment and of course private vehicles owned and driven by construction workers and staff.

It should be noted that construction traffic generated during the Construction Phase tends to be outside of peak hours. (Staff and deliveries arrive before 07:00 and generally depart after 19:00). The traffic generated by the construction phase will not be higher than the peak hour predicted volumes for the Operational Phase. Any specific recommendations/requirements with regard to construction traffic management made by FCC will be adhered to during this phase.

Good construction management practices will be employed such as fencing the site off from the public and neighbouring sites, adequate external/internal signage, secure internal site offices, dedicated construction access points all to ensure the safety construction staff and the public. Appropriate levels of staff parking and compounding will be provided to ensure no potential overflow or haphazard parking in the area. The Site will be able to accommodate employee and visitor parking throughout.

Set construction traffic routes to and from the site will be agreed with FCC prior to the commencement of constructions activities onsite. The time of day permissible for such routes will also be agreed upon and outside of the morning/evening peak hours. Wheel wash facilities will be provided on site to ensure that construction debris will not have an impact on the quality of roads in the Northwood area.

SECTION 7: PARKING PROVISION & STRATEGY

7.1 Overview

A meeting was held with the Roads and Traffic department of Fingal County Council (FCC) on 6th December 2018 to discuss the parking requirements of the Blackwood Square development. It was agreed with FCC that a sustainable approach to parking would be incorporated into the development. The parking strategy utilised is derived from “Sustainable Urban Housing: Design Standards for New Apartments”, which places a strong emphasis on bicycle parking. As per the standards, cycling is a:

“flexible, efficient and attractive transport option for urban living and these guidelines require that this transport mode is fully integrated into the design and operation of all new apartment development schemes.”

The Blackwood Square development is well situated next to high-quality off-road cycling infrastructure and there is an opportunity to maximise the benefit deriving from appropriate cycle parking provision. The proposed development is an integrated area adjacent to existing employment, retail and community facilities. The proposed development is also well situated next to high-quality existing public transport services, as well as planned future public transport upgrades. Additionally, the development is in a “Intermediate Urban Location” as per the standards and therefore car parking should be reduced:

*“In suburban/urban locations served by public transport or close to town centres or employment areas and particularly for housing schemes with more than 45 dwellings per hectare net (18 per acre), **planning authorities must consider a reduced overall car parking standard and apply an appropriate maximum car parking standard.**”*

Due to the close proximity of the proposed development to high-quality off-road cycling infrastructure, as well as numerous existing and future high frequency and high capacity public transport services, the parking strategy for the proposed development is based upon the principles of “Sustainable Urban Housing: Design Standards for New Apartments” in order to further promote sustainable transport modes thus minimising the need for additional car parking.

7.2 Car Sharing Facility

Further to the car parking provision, FCC recommended the provision of 3 no. car parking spaces for a private car sharing company. The benefits of such car sharing services include:

- the reduction of the number of cars on the road and therefore traffic congestion, noise and air pollution;
- frees up land traditionally used for private parking spaces but which may not be used;
- increases use of public transport, walking and cycling as the need for car ownership is reduced; and
- car sharing allows those who cannot afford a car the opportunity to drive, encouraging social inclusivity.

7.3 Car Parking and Cycle Parking

Table 15 summarises the car parking and cycle parking provided within the development. The majority of car/cycle parking will be provided in the basement area. The basement car parking strategy equates to one car parking space per residential unit. The basement cycle parking strategy equates to 1 bicycle parking spaces per residential unit bed space. 70 visitor bicycle parking spaces will be provided on surface level. Additionally, 5 motorcycle parking spaces will be available in the basement level.

Non-residents, utilising the retail area will also be able to avail of the excess car parking provided in the Gulliver’s Retail Park car park. Gulliver’s Retail car park is under the control of the applicant and this car park is to be shared with the Blackwood Square retail units. Any future developments will ensure that retail

car spaces for Blackwood Square will be retained. Non-residents using the retail park will also be able to utilise the visitor parking bicycle spaces located next to the retail areas on the ground floor.

A childcare facility set-down area has also been provided and 5 no. parking spaces are allocated north of the childcare facility area on the ground flood to facilitate servicing, short duration parking and childcare facility pickup / drop off.

Charging points for electric vehicles will be provide at 3 No. surface car parking spaces and provision made for future connections to all surface car parking spaces.

Table 15: Car Parking and Bicycle Parking

	Land Use	Parking Provided	Ratio
Car Parking	Residential Units	331 Car Parking Spaces	1 space per residential unit
		3 Additional Disabled Parking Spaces	1 disabled parking bay per 100 spaces
	Childcare Facility	5 Ground Floor Short-term/Drop-off Car Parking Spaces	1 space per 64m ²
Cycle Parking	Residential Units	690 Basement Cycle Parking Spaces	1 space per residential unit bed space
		70 Visitor Cycle Parking Spaces	1 visitor space per 5 residential units

7.4 Car Ownership Levels

Census 2016 Small Area Population Statistics were analysed in order to determine existing car ownership levels for households in the Northwood area. For the purposes of this analysis, 13 No. locations, which are characterised as being predominately apartment complexes were utilised. These locations mirror closely the proposed Blackwood Square development and are illustrated in Figure 17 below. Small Area populations which comprised mainly office blocks, hotels or industrial areas were excluded from this analysis.



Figure 17: Census 2016 Data Locations (Source: Central Statistics Office)

Table 16 below outlines the total car ownership levels per household for all the existing apartment developments in the Northwood area highlighted in Figure 17:

Table 16: Car Ownership Summary

No. of Households	No. of cars	Ratio - Car space: Residential Unit
1274	1182	0.93

With respect to the application of car parking standards as set out in the Development Plan to the proposed development, there is a requirement for 510 no. residential spaces and 66 no. visitor spaces resulting in 576 no. spaces overall. However with reference to the Design Standards for New Apartments the level of car parking proposed is considered in the context of an intermediate urban location where a reduced level of car parking is required.

In order to determine an appropriate quantum of car parking at this location the review of CSO data was undertaken. As demonstrated in Table 16, the data revealed that the car ownership rate is approximately 1 car per household in the Northwood Area. Providing for this level of car parking to the proposed development results in 331 car parking spaces for the apartments. If any more car parking spaces were introduced, it may encourage an over reliance on single occupancy vehicles, resulting in a negative effect on traffic in the surrounding area. Providing less than one car space per apartment, could lead to haphazard and unofficial parking in the surrounding area, increasing the risk of accidents and traffic congestion.

While no additional car parking spaces are proposed for visitors, visitor car parking demand can be met through shared usage of existing car parking spaces within Gulliver's Retail Park and in the basement as overall car usage declines overtime.

The level of parking proposed for the apartments represents a reduced level of car parking on the site of over 40% based on the Development Plan management standards and is consistent with an intermediate urban location where a reduced level of car parking is required.

SECTION 8: SUMMARY & CONCLUSION

This report has been designed to specifically address potential traffic issues associated with the proposed development. In doing so, it has considered:

- Trip generation rates to/from the proposed development;
- Proposed access arrangements;
- Existing and future road network capacity;
- Existing and future road safety assessment.

The proposed development will consist of 331 apartments in four separate blocks, with mixed used commercial units, a gym area and a childcare facility, all over basement car parking, and all associated site works including roads, footpaths, landscaping, site services, SuDS measures and sundry related works.

A turning movement traffic count was undertaken at seven junctions near the subject site. The vehicle turning movement surveys were undertaken on Tuesday 12th February 2019. The counts were carried out over the 12-hour period 07:00 hours to 19:00 hours including both the morning and evening peak periods. The morning peak hour was identified as 08:00-09:00 for all junctions. The evening peak hour was identified as 17:00-18:00 for all junctions. Data was collected in 15-minute intervals.

It was determined that Junction 2) Ballymun Road (R108) / Northwood Avenue, Junction 5) Northwood Ave / Northwood Road, Junction 6) Santry Ave / Northwood Road and Junction 7) Northwood Ave / Swords Road (R138) were the key junctions to be modelled.

The analysis demonstrated that Junction 2) Ballymun Road (R108) / Northwood Avenue currently operates just over the normal design threshold during the morning and evening peak hours considered. Junction 7) Northwood Ave / Swords Road (R138) will exceed the normal design threshold on the Swords Road arm during the evening peak hour. Junction 5) Northwood Ave / Northwood Road and Junction 6) Santry Ave / Northwood Road currently operate efficiently below the design threshold.

In the future, the junctions will not operate efficiently in either the “without” and “**with**” the development scenarios. Any future traffic growth, irrespective of the subject development, will therefore result in an impact to the operation of the junction. It is noted that the 2036 analysis does not include the likely improvements in the public transport services (MetroLink and BusConnects).

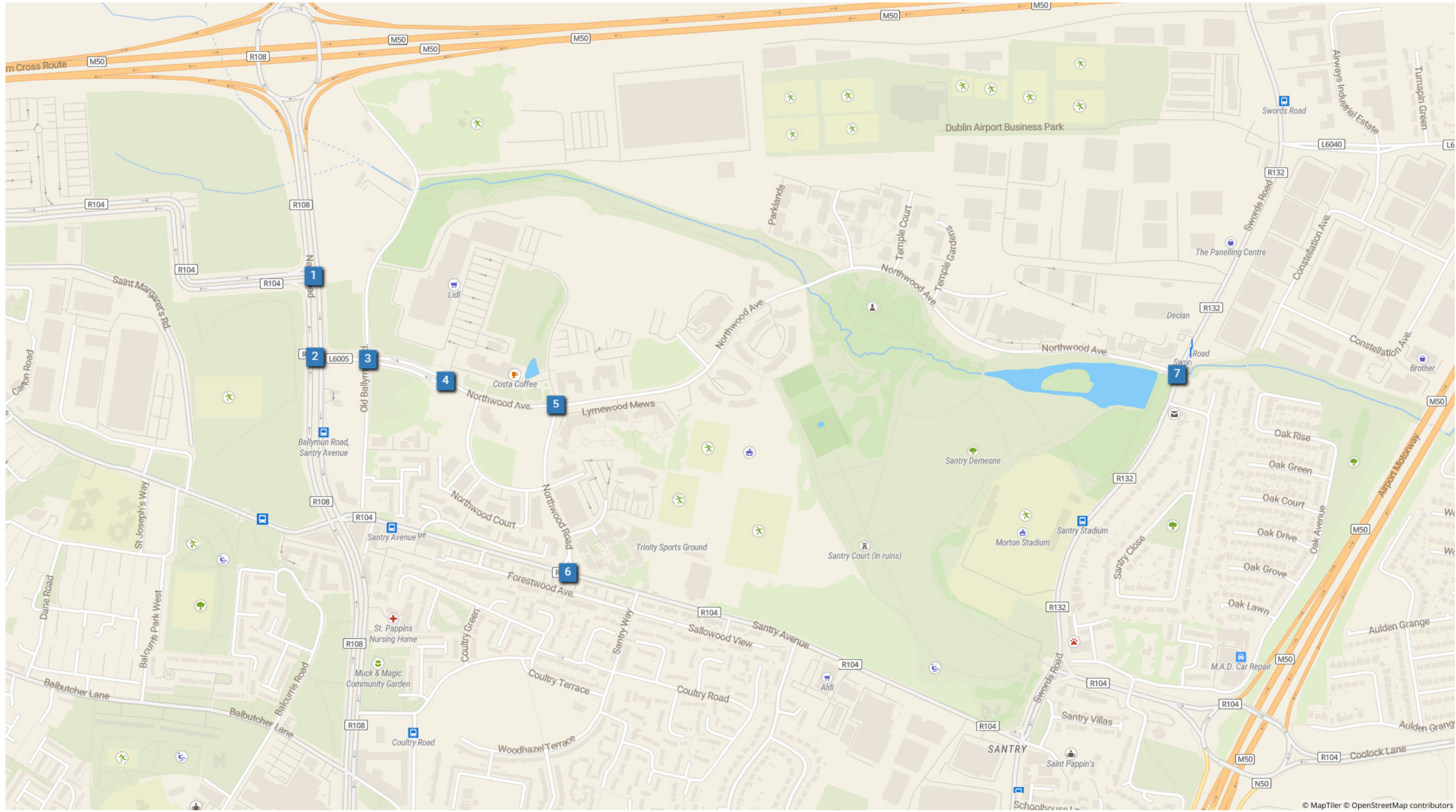
It is noted that Fingal plan to upgrade Junction 3) Northwood Avenue / Old Ballymun Road to incorporate SCATS. Upgrading this junction to SCATS will allow the junction to control the traffic arriving from Northwood to the Ballymun Road. The junctions will better calculate and adapt the timing of traffic signals in the network allowing the junction to operate efficiently, creating an overall positive impact.

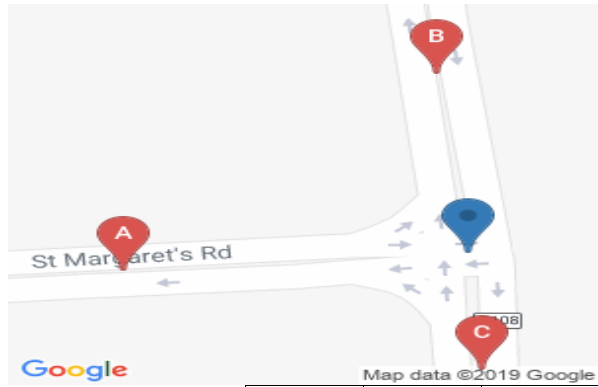
This further emphasises the need for an improved public transport network as outlined in Section 2.3, such as MetroLink and BusConnects, regardless of the Blackwood Square development. Additionally, further investments are needed in cycling infrastructure not just in Northwood, but the Greater Dublin Area. This will encourage a greater number of Northwood residents to leave the car at home and choose sustainable transport modes.

The parking strategy utilised is derived from “Sustainable Urban Housing: Design Standards for New Apartments”, which places a strong emphasis on bicycle parking. The basement car parking strategy equates to one car parking space per residential unit. The basement cycle parking strategy equates to roughly 1 bicycle parking space per residential unit bed space.

The study concludes that the proposed development as described herein, does not pose any significant residual risks from a traffic and safety perspective.

Appendix 1- Traffic Count Data





IDASO

Survey Name: 029 19025 Northwood
Site: 1
Location: St Margaret's Rd
Date: 12-Feb-2019

TIME	A1	A2	A3	B1	B2	B3	B4	C1	C2	C3
07:00	5	25	25	200	120	25	10	5	35	30
07:15	10	25	25	180	130	20	25	10	50	45
07:30	5	25	25	220	200	25	20	5	65	50
07:45	5	35	30	170	160	55	35	15	90	70
08:00	5	40	35	300#	300#	35	50	5	125#	90
08:15	15	35	35	300#	300#	45	15	5	80	55
08:30	5	40	30	300#	300#	40	35	5	85	50
08:45	5	45	30	200	240	40	25	0	125#	70
09:00	5	30	25	300#	300#	30	20	0	125#	100
09:15	0	25	25	180	120	15	15	10	115'*	125#
09:30	0	15	20	170	130	20	30	10	75	50
09:45	0	20	15	180	140	25	20	15	50	40
10:00	15	10	15	150	100	30	20	5	55	35
10:15	15	15	10	75	50	40	50	0	50	30
10:30	5	25	15	100	75	35	40	0	40	35
10:45	5	20	20	90	50	30	30	0	50	30
11:00	5	20	20	100	80	35	50	5	40	30
11:15	10	20	20	30	30	20	45	0	50	25
11:30	15	15	15	45	35	25	35	5	50	40
11:45	20	55	45	120	80	30	40	5	45	35
12:00	5	55	45	110	70	30	30	0	125#	125#
12:15	10	35	40	45	35	25	45	0	125#	70
12:30	15	45	40	50	30	45	50	0	55	40
12:45	10	55	55	45	30	50	65	15	40	30

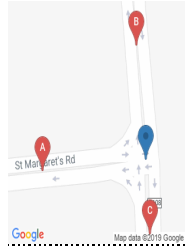
TIME	A1	A2	A3	B1	B2	B3	B4	C1	C2	C3
13:00	0	20	25	120	70	25	55	0	50	40
13:15	15	25	35	120	80	35	50	0	80	45
13:30	15	40	40	90	45	50	30	0	50	35
13:45	20	60	50	55	40	45	55	15	40	35
14:00	10	40	40	60	40	35	40	0	45	40
14:15	0	35	35	90	50	40	35	5	45	35
14:30	0	20	25	90	90	25	25	0	30	30
14:45	25	25	35	60	40	30	40	0	55	45
15:00	40	25	20	130	70	55	35	0	45	40
15:15	55	40	35	120	80	30	40	0	125#	50
15:30	35	45	40	140	100	25	25	0	55	45
15:45	20	30	35	120	90	40	50	0	70	45
16:00	55	40	45	130	100	20	20	0	65	40
16:15	45	50	45	150	120	35	30	0	125#	55
16:30	50	35	30	100	110	25	25	0	80	40
16:45	60	50	60	130	100	30	30	0	65	60
17:00	0	60	60	90	70	45	30	10	125#	125#
17:15	75	50	40	110	90	35	25	15	125#	125#
17:30	35	35	40	160	140	25	20	0	125#	125#
17:45	35	30	30	95	70	40	30	10	125#	125#
18:00	30	40	35	60	45	35	30	0	125#	125#
18:15	50	30	30	90	65	40	30	0	55	40
18:30	25	35	40	45	30	30	30	10	60	40
18:45	25	40	30	40	30	25	25	5	40	40

Queue's are measured in meters

Cannot be seen from camera

- + Signifies queue stretches to a minimum length of x and beyond the view of the camera
- # Signifies queue stretches to the next significant junction
- * Indicates an estimated queue length due to obscured vision.

Queue lengths are compiled from CCTV observations and are therefore subject to the limitations of the camera view.



IDASO

Survey Name: 029 19025 Northwood
Site: 1
Location: St Margaret's Rd
Date: 12-Feb-2019

Large data table with columns for Time, Direction (A=>A, A=>B, A=>C, B=>A, B=>B, B=>C, C=>A, C=>B, C=>C), and various traffic volume metrics (P/C, M/C, CAR, LGV, OGV1, OGV2, PSV, TOT, PCU).



IDASO

Survey Name: 029 19025 Northwood
Site: 2
Location: R108 / Northwood Ave
Date: 12-Feb-2019

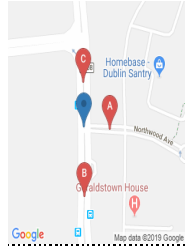
TIME	A1	A2	A3	B1	B2	B3	B4	C1	C2	C3
07:00	15	20	25	10	30	15	40	30	50	50
07:15	20	60#	40	5	20	0	25	30	50	40
07:30	15	45	25	0	20	15	30	40	40	60
07:45	20	15	30	30	10	0	20	30	125#	70
08:00	15	60#	40	10	50	10	20	20	80	125#
08:15	15	60#	40	25	20	10	25	30	125#	125#
08:30	10	60#	40	5	30	5	80	40	125#	60
08:45	15	30	20	15	30	15	80	60	125#	50
09:00	10	60#	40	10	35	30	50	20	30	20
09:15	10	50	30	15	30	15	50	40	100	30
09:30	20	60#	40	15	35	5	30	20	50	40
09:45	20	60#	30	15	10	0	50	20	30	40
10:00	15	60#	30	5	20	10	20	25	25	30
10:15	20	60#	35	5	15	5	25	20	30	40
10:30	15	60#	25	0	15	5	20	15	20	25
10:45	15	30	30	10	15	5	10	20	25	30
11:00	10	40	35	15	20	5	20	30	15	10
11:15	25	40	35	10	20	5	15	15	20	20
11:30	15	40	20	15	20	10	25	10	25	30
11:45	15	30	20	10	20	15	20	15	20	20
12:00	20	30	20	15	20	10	35	10	15	20
12:15	20	50	15	0	20	25	20	15	30	20
12:30	10	40	20	10	20	10	30	20	20	25
12:45	25	50	30	10	20	10	25	30	15	30

TIME	A1	A2	A3	B1	B2	B3	B4	C1	C2	C3
13:00	25	30	40	5	30	20	45	30	20	30
13:15	20	50	30	20	25	20	20	15	20	15
13:30	25	60#	30	15	35	20	30	15	30	25
13:45	30	60#	20	15	30	25	110	25	20	20
14:00	50	60#	30	20	30	10	30	10	30	20
14:15	20	60#	40	10	40	5	30	15	30	20
14:30	30	60#	40	15	30	25	30	15	20	15
14:45	30	60#	30	10	20	5	25	15	30	125#
15:00	20	60#	25	10	25	20	30	25	50	30
15:15	40	60#	25	0	20	10	30	10	40	30
15:30	30	60#	40	10	30	15	35	15	25	15
15:45	15	60#	45	5	40	10	30	10	20	15
16:00	50	60#	35	15	110	15	30	5	20	25
16:15	30	60#	30	5	20	20	40	15	20	15
16:30	20	50	40	5	110	15	30	15	30	20
16:45	20	60#	40	15	115	15	35	20	25	35
17:00	50	60#	40	5	150	30	40	15	20	15
17:15	60#	60#	40	15	165	130	30	10	15	10
17:30	60#	60#	40	10	130	20	40	15	35	25
17:45	40	60#	30	15	70	20	40	15	10	15
18:00	30	60#	20	10	100	20	30	15	40	20
18:15	20	60#	40	5	120	15	35	10	20	15
18:30	20	50	30	5	30	15	10	10	15	10
18:45	20	50	30	15	20	15	15	5	15	35

Queue's are measured in meters

- Cannot be seen from camera
- + Signifies queue stretches to a minimum length of x and beyond the view of the camera
- # Signifies queue stretches to the next significant junction
- * Indicates an estimated queue length due to obscured vision.

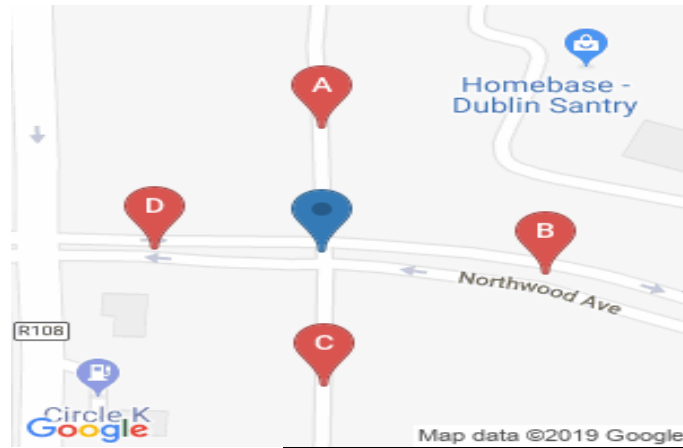
Queue lengths are compiled from CCTV observations and are therefore subject to the limitations of the camera view.



IDASO

Survey Name: 029 19025 Northwood
Site: 2
Location: R108 / Northwood Ave
Date: 12-Feb-2019

Large data table with columns for Time, P/C, M/C, CAR, LGV, OGV1, OGV2, PSV, TOT, and various vehicle counts for different directions (A=>A, A=>B, A=>C, B=>A, B=>B, B=>C, C=>A, C=>B, C=>C).



IDASO

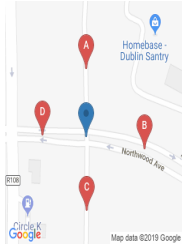
Survey Name: 029 19025 Northwood
Site: 3
Location: Northwood Ave
Date: 12-Feb-2019

TIME	A1	A2	B1	B2	C1	D1	D2	D3		TIME	A1	A2	B1	B2	C1	D1	D2	D3
07:00	0	10	40	60	5	50	30	10		13:00	0	15	40	40	5	60#	50	10
07:15	0	25	75	75	5	40	40	10		13:15	0	20	30	55	10	50	50	10
07:30	10	15	60	75	10	60#	60#	10		13:30	10	25	35	75	5	40	50	20
07:45	15	15	20	30	5	60#	60#	5		13:45	5	25	40	75	10	60#	60#	15
08:00	15	20	30	50	5	40	60#	5		14:00	10	30	40	60	10	60#	50	15
08:15	10	30	70	70	10	60#	60#	5		14:15	0	20	70	40	5	50	40	20
08:30	10	20	20	20	15	60#	60#	5		14:30	0	20	70	30	10	60#	60#	5
08:45	0	25	20	20	15	60#	60#	10		14:45	0	20	30	70	15	50	50	5
09:00	0	30	20	70	10	60#	60#	10		15:00	5	25	30	40	5	30	50	5
09:15	0	30	15	25	10	60#	60#	5		15:15	0	45	30	50	10	30	50	10
09:30	0	15	15	35	10	60#	60#	5		15:30	0	15	20	60	25	60#	50	5
09:45	5	20	20	40	10	60#	60#	5		15:45	0	25	60	95	10	40	50	10
10:00	5	20	15	30	5	50	60#	10		16:00	0	30	40	120#	25	30	40	5
10:15	0	35	10	40	5	50	40	5		16:15	10	15	25	120#	10	30	60#	5
10:30	0	15	30	55	15	25	35	0		16:30	5	15	30	70	5	40	30	10
10:45	0	10	25	45	5	50	40	5		16:45	5	10	40	120#	15	50	40	10
11:00	20	20	20	60	5	60#	60#	10		17:00	5	45	50	120#	5	40	50	10
11:15	0	35	30	70	15	60#	40	10		17:15	10	15	70	120#	15	60#	60#	10
11:30	10	15	20	40	10	40	35	10		17:30	0	25	40	120#	10	60#	60#	10
11:45	0	20	30	20	5	30	50	5		17:45	0	20	50	120#	15	40	60#	5
12:00	0	20	20	30	5	40	50	5		18:00	0	30	40	120#	10	30	60#	5
12:15	0	20	40	60	15	50	60#	5		18:15	0	20	30	70	5	10	30	5
12:30	5	25	30	50	10	50	40	10		18:30	0	15	30	60	5	50	40	5
12:45	0	15	25	40	10	60#	60#	5		18:45	0	25	20	35	10	20	40	5

Queue's are measured in meters

- Cannot be seen from camera
- + Signifies queue stretches to a minimum length of x and beyond the view of the camera
- # Signifies queue stretches to the next significant junction
- * Indicates an estimated queue length due to obscured vision.

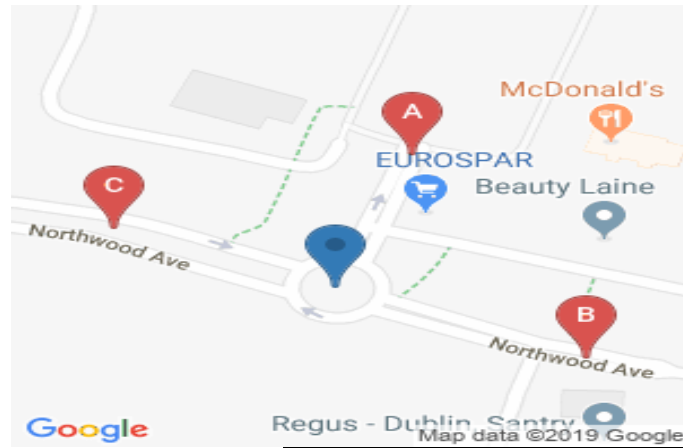
Queue lengths are compiled from CCTV observations and are therefore subject to the limitations of the camera view.



IDASO

Survey Name: 029 19025 Northwood
Site: 3
Location: Northwood Ave
Date: 12-Feb-2019

Large data table with columns for Time, P/C, M/C, CAR, LGV, OGV1, OGV2, PSV, TOT, and various vehicle counts for different directions (A=>A, A=>B, A=>C, A=>D, B=>A, B=>B, B=>C, B=>D, C=>A, C=>B).



IDASO

Survey Name: 029 19025 Northwood
Site: 4
Location: Northwood Ave
Date: 12-Feb-2019

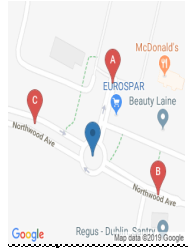
TIME	A1	B1	B2	C1	C2
07:00	5	10	0	5	0
07:15	10	5	5	20	15
07:30	5	15	10	15	25
07:45	10	10	0	10	15
08:00	10	10	10	5	25
08:15	10	5	5	15	75
08:30	20	5	10	35	95
08:45	25	5	10	30	80
09:00	15	10	10	25	70
09:15	20	10	5	35	50
09:30	15	10	10	20	15
09:45	10	10	10	25	30
10:00	15	5	5	15	20
10:15	15	5	15	10	20
10:30	10	10	10	15	20
10:45	15	10	10	10	20
11:00	30	10	10	10	15
11:15	20	5	10	5	10
11:30	20	15	5	10	15
11:45	15	10	15	15	10
12:00	20	10	5	15	20
12:15	15	10	10	20	30
12:30	15	15	10	15	10
12:45	20	15	15	20	25

TIME	A1	B1	B2	C1	C2
13:00	45	10	15	50	85
13:15	45	15	10	30	75
13:30	20	15	15	20	45
13:45	30	10	15	20	30
14:00	35	15	5	10	20
14:15	25	10	5	15	20
14:30	10	10	15	15	15
14:45	20	10	15	25	10
15:00	20	15	15	20	10
15:15	20	15	15	15	10
15:30	25	20	10	10	15
15:45	25	15	10	30	20
16:00	30	10	15	15	40
16:15	15	15	15	20	15
16:30	30	10	15	10	10
16:45	30	35	30	15	40
17:00	45	40	70	20	30
17:15	45	20	65	10	15
17:30	45	45	85	20	20
17:45	30	50	30	10	15
18:00	15	20	15	15	15
18:15	20	20	10	10	15
18:30	20	10	10	15	25
18:45	15	15	10	10	15

Queue's are measured in meters

- Cannot be seen from camera
- + Signifies queue stretches to a minimum length of x and beyond the view of the camera
- # Signifies queue stretches to the next significant junction
- * Indicates an estimated queue length due to obscured vision.

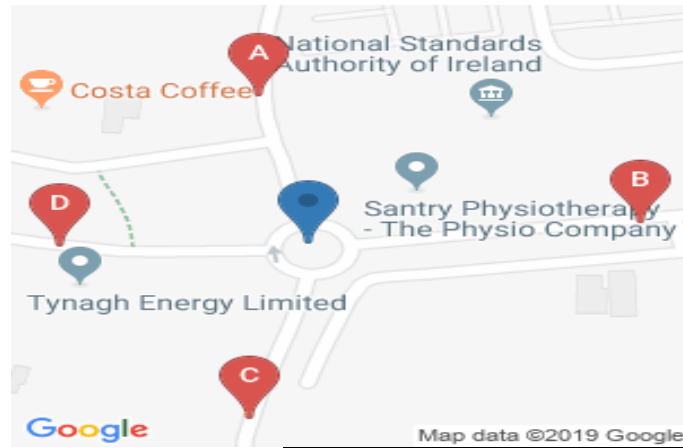
Queue lengths are compiled from CCTV observations and are therefore subject to the limitations of the camera view.



IDASO

Survey Name: 029 19025 Northwood
Site: 4
Location: Northwood Ave
Date: 12-Feb-2019

Table with columns for Time, P/C, M/C, CAR, LGV, OGV1, OGV2, PSV, TOT, and PCU for various directions (A=>A, A=>B, A=>C, B=>A, B=>B, B=>C, C=>A, C=>B, C=>C). Rows represent time intervals from 07:00 to 18:45.



IDASO

Survey Name: 029 19025 Northwood
Site: 5
Location: Northwood Ave
Date: 12-Feb-2019

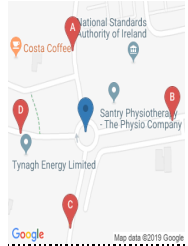
TIME	A1	A2	B1	B2	C1	C2	D1	D2
07:00	5	0	5	0	5	5	5	5
07:15	10	0	0	0	5	10	10	10
07:30	10	5	5	5	10	10	10	15
07:45	5	0	0	5	5	10	15	20
08:00	10	0	0	5	10	5	10	15
08:15	10	0	0	0	10	10	15	25
08:30	5	0	5	0	20	0	10	15
08:45	5	0	10	5	10	10	15	15
09:00	5	10	5	5	5	10	10	20
09:15	5	10	0	5	5	15	15	30
09:30	0	5	5	10	5	10	10	5
09:45	10	5	5	5	5	5	15	5
10:00	10	5	5	5	5	5	10	5
10:15	5	0	10	5	5	5	10	5
10:30	5	0	10	5	5	10	10	5
10:45	5	0	10	0	5	10	5	10
11:00	10	5	5	0	5	5	5	0
11:15	10	5	10	5	10	0	15	0
11:30	5	0	0	0	15	10	15	5
11:45	5	0	15	10	5	10	5	5
12:00	5	5	5	10	5	5	5	5
12:15	5	5	5	5	10	5	0	0
12:30	5	5	5	10	5	10	5	10
12:45	0	15	5	10	20	5	10	15

TIME	A1	A2	B1	B2	C1	C2	D1	D2
13:00	5	15	0	10	10	10	15	10
13:15	10	0	10	0	10	20	5	15
13:30	10	5	5	5	10	10	5	15
13:45	5	5	0	0	10	5	10	15
14:00	5	0	5	10	15	15	15	5
14:15	5	10	5	5	5	0	10	5
14:30	5	5	10	0	10	5	5	10
14:45	5	5	5	0	10	10	10	20
15:00	5	5	5	15	5	10	5	15
15:15	5	10	10	5	10	10	5	10
15:30	10	5	0	0	10	5	10	5
15:45	10	0	10	10	5	5	5	10
16:00	5	5	0	5	5	5	5	5
16:15	10	5	5	5	10	10	20	10
16:30	10	10	0	20	10	15	10	25
16:45	10	15	5	15	5	15	20	15
17:00	10	10	15	10	10	10	10	25
17:15	5	10	15	15	10	25	10	10
17:30	15	10	5	25	10	15	15	20
17:45	5	10	5	10	15	20	5	10
18:00	15	10	10	5	5	10	10	45
18:15	5	5	5	15	15	20	15	10
18:30	10	5	0	10	10	10	10	10
18:45	10	5	10	5	5	5	5	15

Queue's are measured in meters

- Cannot be seen from camera
- + Signifies queue stretches to a minimum length of x and beyond the view of the camera
- # Signifies queue stretches to the next significant junction
- * Indicates an estimated queue length due to obscured vision.

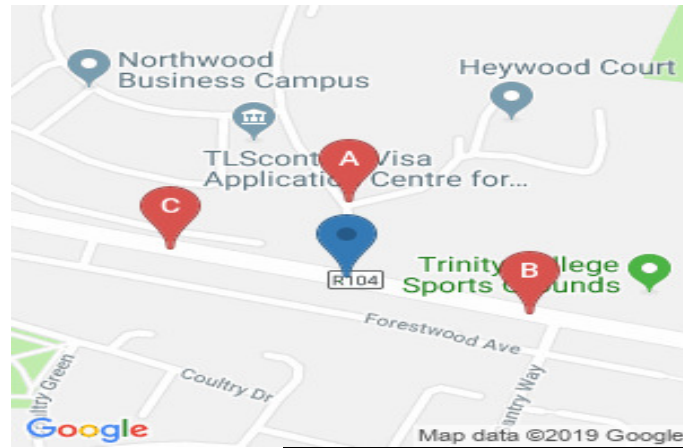
Queue lengths are compiled from CCTV observations and are therefore subject to the limitations of the camera view.



IDASO

Survey Name: 029 19025 Northwood
Site: 5
Location: Northwood Ave
Date: 12-Feb-2019

Large data table with columns for TIME, P/C, M/C, CAR, LGV, OGV1, OGV2, PSV, TOT, and various PCU, P/C, M/C, CAR, LGV, OGV1, OGV2, PSV, TOT sub-columns for categories A=>A, A=>B, A=>C, A=>D, B=>A, B=>B, B=>C, B=>D, and C=>A, C=>B.



IDASO

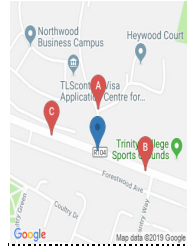
Survey Name: 029 19025 Northwood
Site: 6
Location: Forestwood Ave / Santry Ave
Date: 12-Feb-2019

TIME	A1	A2	B1	B2	C1	C2	TIME	A1	A2	B1	B2	C1	C2
07:00	10	5	15	5	10	15	13:00	20	10	35	20	10	110
07:15	10	10	15	10	10	20	13:15	25	10	15	25	5	60
07:30	15	20	30	15	10	30	13:30	20	10	25	20	5	40
07:45	15	20	30	10	20	35	13:45	20	5	35	10	15	40
08:00	25	20	30	30	5	50	14:00	10	15	55	15	10	60
08:15	10	20	40	25	10	60	14:15	30	5	65	15	5	50
08:30	45	10	30	25	10	130	14:30	25	5	40	10	10	55
08:45	25	10	25	10	30	130	14:45	10	15	30	20	15	65
09:00	20	15	40	30	55	120	15:00	15	10	35	30	5	100
09:15	15	10	30	20	15	60	15:15	20	15	30	20	10	65
09:30	10	10	35	10	10	55	15:30	25	5	50	15	10	45
09:45	15	5	55	20	10	50	15:45	20	15	30	15	20	55
10:00	10	5	40	5	10	60	16:00	20	15	55	30	10	65
10:15	20	5	20	15	15	30	16:15	10	15	65	20	10	50
10:30	25	10	30	15	15	40	16:30	20	35	65	15	10	60
10:45	15	30	40	10	5	50	16:45	40	35	60	30	10	100
11:00	15	5	30	10	5	35	17:00	30	50	75	55	40	130+
11:15	10	10	20	15	5	30	17:15	40	40	110	45	15	100
11:30	15	10	30	15	5	40	17:30	30	60	120+	30	10	60
11:45	15	10	50	15	5	40	17:45	40	30	90	30	20	100
12:00	20	5	30	15	5	60	18:00	35	25	55	30	15	55
12:15	20	15	10	5	10	40	18:15	25	25	60	25	15	65
12:30	20	15	30	15	5	35	18:30	25	15	45	35	10	55
12:45	15	5	50	15	20	120	18:45	15	20	15	20	10	50

Queue's are measured in meters

- Cannot be seen from camera
- + Signifies queue stretches to a minimum length of x and beyond the view of the camera
- # Signifies queue stretches to the next significant junction
- * Indicates an estimated queue length due to obscured vision.

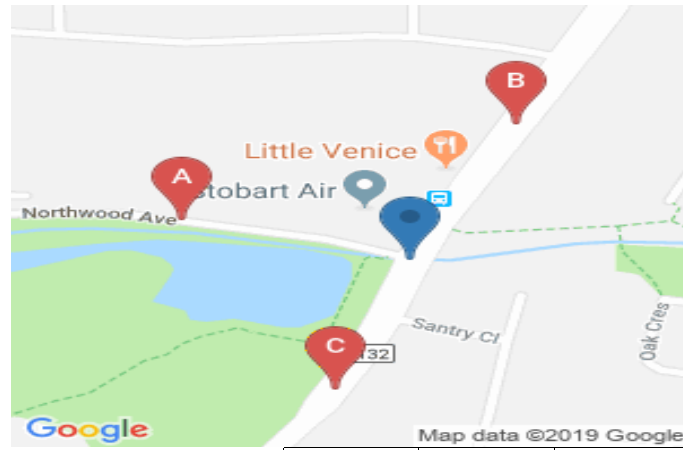
Queue lengths are compiled from CCTV observations and are therefore subject to the limitations of the camera view.



IDASO

Survey Name: 029 19025 Northwood
Site: 6
Location: Forestwood Ave / Santry Ave
Date: 12-Feb-2019

Large data table with columns for Time, Direction (A=>A, A=>B, A=>C, B=>A, B=>B, B=>C, C=>A, C=>B, C=>C), and various traffic metrics (P/C, M/C, CAR, LGV, OGV1, OGV2, PSV, TOT, PCU).



IDASO

Survey Name: 029 19025 Northwood
Site: 7
Location: Northwood Ave / Swords Rd
Date: 12-Feb-2019

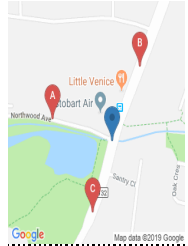
TIME	A1	A2	B1	B2	B3	C1	C2
07:00	25	30	20	25	10	15	25
07:15	25	30	40	50	15	25	40
07:30	20	45	45	150	20	35	100
07:45	45	50	40	70	50	30	110
08:00	35	40	30	130	40	45	120
08:15	55	45	50	180+	30	35	70
08:30	60	55	30	80	50	50	200+
08:45	85	50	20	70	45	40	200+
09:00	40	30	15	150	30	60	120
09:15	60	55	20	100	25	50	200+
09:30	50	30	15	65	20	40	60
09:45	30	25	15	70	20	30	110
10:00	30	25	30	55	15	45	60
10:15	35	30	15	50	25	40	80
10:30	30	35	25	130	15	25	40
10:45	30	50	15	60	10	20	50
11:00	20	35	20	55	25	20	45
11:15	30	40	20	100	30	45	70
11:30	50	35	15	70	15	30	100
11:45	35	40	20	80	20	25	80
12:00	40	25	10	80	30	20	50
12:15	30	50	20	90	20	30	100
12:30	30	40	30	180+	50	30	60
12:45	25	30	30	180+	40	30	110

TIME	A1	A2	B1	B2	B3	C1	C2
13:00	30	30	30	180+	60	30	105
13:15	30	50	20	135	20	50	80
13:30	50	45	30	130	20	30	120
13:45	35	25	20	100	20	30	130
14:00	30	50	15	145	20	60	80
14:15	35	55	15	130	20	35	70
14:30	40	40	20	70	25	30	120
14:45	45	50	15	60	35	45	70
15:00	35	30	15	70	25	40	90
15:15	50	30	30	55	25	30	80
15:30	30	45	20	120	60	45	110
15:45	30	20	20	75	15	40	90
16:00	35	45	20	180+	30	40	50
16:15	40	100	20	130	30	30	60
16:30	40	125	15	150	30	40	80
16:45	35	170	10	150	25	30	65
17:00	50	200+	10	180+	55	50	170
17:15	50	200+	150	180+	70	50	170
17:30	55	200+	40	180+	55	40	120
17:45	50	200+	75	180+	70	40	130
18:00	60	150	40	180+	50	60	130
18:15	30	40	50	135	20	30	70
18:30	40	60	20	90	30	20	30
18:45	25	40	15	60	20	20	40

Queue's are measured in meters

- Cannot be seen from camera
- + Signifies queue stretches to a minimum length of x and beyond the view of the camera
- # Signifies queue stretches to the next significant junction
- * Indicates an estimated queue length due to obscured vision.

Queue lengths are compiled from CCTV observations and are therefore subject to the limitations of the camera view.



IDASO

Survey Name: 029 19025 Northwood
Site: 7
Location: Northwood Ave / Swords Rd
Date: 12-Feb-2019

Table with columns for Time, P/C, M/C, CAR, LGV, OGV1, OGV2, PSV, TOT, and various PCU values for different vehicle types and directions (A=>A, A=>B, A=>C, B=>A, B=>B, B=>C, C=>A, C=>B, C=>C).

Appendix 2- TRICS Output Files

TRIP RATE for Land Use 03 - RESIDENTIAL/C - FLATS PRIVATELY OWNED
VEHICLES

Calculation factor: 1 TOTBED

Estimated TRIP rate value per 697 TOTBED shown in shaded columns

BOLD print indicates peak (busiest) period

Time Range	ARRIVALS				DEPARTURES				TOTALS			
	No. Days	Ave. TOTBED	Trip Rate	Estimated Trip Rate	No. Days	Ave. TOTBED	Trip Rate	Estimated Trip Rate	No. Days	Ave. TOTBED	Trip Rate	Estimated Trip Rate
00:00 - 01:00												
01:00 - 02:00												
02:00 - 03:00												
03:00 - 04:00												
04:00 - 05:00												
05:00 - 06:00												
06:00 - 07:00												
07:00 - 08:00	44	130	0.023	16.121	44	130	0.087	60.821	44	130	0.110	76.942
08:00 - 09:00	44	130	0.028	19.175	44	130	0.106	74.133	44	130	0.134	93.308
09:00 - 10:00	44	130	0.034	23.938	44	130	0.051	35.296	44	130	0.085	59.234
10:00 - 11:00	44	130	0.028	19.541	44	130	0.038	26.502	44	130	0.066	46.043
11:00 - 12:00	44	130	0.034	23.693	44	130	0.037	26.136	44	130	0.071	49.829
12:00 - 13:00	44	130	0.046	31.998	44	130	0.042	29.556	44	130	0.088	61.554
13:00 - 14:00	44	130	0.042	28.945	44	130	0.044	30.777	44	130	0.086	59.722
14:00 - 15:00	44	130	0.042	29.067	44	130	0.042	29.189	44	130	0.084	58.256
15:00 - 16:00	44	130	0.053	36.639	44	130	0.036	25.037	44	130	0.089	61.676
16:00 - 17:00	44	130	0.065	45.310	44	130	0.039	26.991	44	130	0.104	72.301
17:00 - 18:00	44	130	0.098	68.515	44	130	0.041	28.456	44	130	0.139	96.971
18:00 - 19:00	44	130	0.087	60.699	44	130	0.043	29.678	44	130	0.130	90.377
19:00 - 20:00												
20:00 - 21:00												
21:00 - 22:00												
22:00 - 23:00												
23:00 - 24:00												
Total Rates:			0.580	403.641			0.606	422.572			1.186	826.213

This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: $COUNT/TRP*FACT$. Trip rates are then rounded to 3 decimal places.

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Parameter summary

Trip rate parameter range selected:	10 - 780 (units:)
Survey date date range:	01/01/11 - 05/06/18
Number of weekdays (Monday-Friday):	44
Number of Saturdays:	0
Number of Sundays:	0
Surveys automatically removed from selection:	1
Surveys manually removed from selection:	0

This section displays a quick summary of some of the data filtering selections made by the TRICS® user. The trip rate calculation parameter range of all selected surveys is displayed first, followed by the range of minimum and maximum survey dates selected by the user. Then, the total number of selected weekdays and weekend days in the selected set of surveys are show. Finally, the number of survey days that have been manually removed from the selected set outside of the standard filtering procedure are displayed.

TRIP RATE for Land Use 01 - RETAIL/I - SHOPPING CENTRE - LOCAL SHOPS
VEHICLES

Calculation factor: 100 sqm

Estimated TRIP rate value per 1006 SQM shown in shaded columns

BOLD print indicates peak (busiest) period

Time Range	ARRIVALS				DEPARTURES				TOTALS			
	No. Days	Ave. GFA	Trip Rate	Estimated Trip Rate	No. Days	Ave. GFA	Trip Rate	Estimated Trip Rate	No. Days	Ave. GFA	Trip Rate	Estimated Trip Rate
00:00 - 01:00												
01:00 - 02:00												
02:00 - 03:00												
03:00 - 04:00												
04:00 - 05:00												
05:00 - 06:00												
06:00 - 07:00	1	540	1.296	13.041	1	540	1.296	13.041	1	540	2.592	26.082
07:00 - 08:00	10	1180	2.204	22.172	10	1180	1.856	18.675	10	1180	4.060	40.847
08:00 - 09:00	10	1180	2.899	29.164	10	1180	2.357	23.707	10	1180	5.256	52.871
09:00 - 10:00	10	1180	3.806	38.289	10	1180	3.213	32.320	10	1180	7.019	70.609
10:00 - 11:00	10	1180	3.806	38.289	10	1180	3.408	34.281	10	1180	7.214	72.570
11:00 - 12:00	10	1180	4.145	41.700	10	1180	4.399	44.258	10	1180	8.544	85.958
12:00 - 13:00	10	1180	5.289	53.212	10	1180	5.044	50.739	10	1180	10.333	103.951
13:00 - 14:00	10	1180	4.662	46.902	10	1180	4.654	46.816	10	1180	9.316	93.718
14:00 - 15:00	10	1180	4.408	44.343	10	1180	4.493	45.196	10	1180	8.901	89.539
15:00 - 16:00	10	1180	3.933	39.568	10	1180	4.196	42.212	10	1180	8.129	81.780
16:00 - 17:00	10	1180	4.230	42.553	10	1180	3.993	40.165	10	1180	8.223	82.718
17:00 - 18:00	10	1180	4.247	42.723	10	1180	4.789	48.181	10	1180	9.036	90.904
18:00 - 19:00	10	1180	4.569	45.964	10	1180	4.815	48.437	10	1180	9.384	94.401
19:00 - 20:00	8	1397	3.938	39.620	8	1397	4.225	42.502	8	1397	8.163	82.122
20:00 - 21:00	8	1397	2.694	27.104	8	1397	3.070	30.886	8	1397	5.764	57.990
21:00 - 22:00	6	1103	2.812	28.287	6	1103	3.099	31.176	6	1103	5.911	59.463
22:00 - 23:00												
23:00 - 24:00												
Total Rates:			58.938	592.931			58.907	592.592			117.845	1185.523

This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: $COUNT/TRP*FACT$. Trip rates are then rounded to 3 decimal places.

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Parameter summary

Trip rate parameter range selected:	260 - 3837 (units: sqm)
Survey date date range:	01/01/11 - 23/03/18
Number of weekdays (Monday-Friday):	10
Number of Saturdays:	0
Number of Sundays:	0
Surveys automatically removed from selection:	0
Surveys manually removed from selection:	0

This section displays a quick summary of some of the data filtering selections made by the TRICS® user. The trip rate calculation parameter range of all selected surveys is displayed first, followed by the range of minimum and maximum survey dates selected by the user. Then, the total number of selected weekdays and weekend days in the selected set of surveys are show. Finally, the number of survey days that have been manually removed from the selected set outside of the standard filtering procedure are displayed.

TRIP RATE for Land Use 04 - EDUCATION/D - NURSERY
VEHICLES

Calculation factor: 100 sqm

Estimated TRIP rate value per 321 SQM shown in shaded columns

BOLD print indicates peak (busiest) period

Time Range	ARRIVALS				DEPARTURES				TOTALS			
	No. Days	Ave. GFA	Trip Rate	Estimated Trip Rate	No. Days	Ave. GFA	Trip Rate	Estimated Trip Rate	No. Days	Ave. GFA	Trip Rate	Estimated Trip Rate
00:00 - 01:00												
01:00 - 02:00												
02:00 - 03:00												
03:00 - 04:00												
04:00 - 05:00												
05:00 - 06:00												
06:00 - 07:00	2	328	0.152	0.489	2	328	0.000	0.000	2	328	0.152	0.489
07:00 - 08:00	19	383	1.870	6.002	19	383	1.017	3.266	19	383	2.887	9.268
08:00 - 09:00	19	383	4.235	13.594	19	383	3.286	10.548	19	383	7.521	24.142
09:00 - 10:00	19	383	2.159	6.929	19	383	2.104	6.753	19	383	4.263	13.682
10:00 - 11:00	19	383	0.577	1.854	19	383	0.454	1.456	19	383	1.031	3.310
11:00 - 12:00	19	383	0.839	2.692	19	383	0.577	1.854	19	383	1.416	4.546
12:00 - 13:00	19	383	1.389	4.458	19	383	1.746	5.605	19	383	3.135	10.063
13:00 - 14:00	19	383	1.031	3.310	19	383	1.347	4.325	19	383	2.378	7.635
14:00 - 15:00	19	383	0.880	2.825	19	383	0.811	2.604	19	383	1.691	5.429
15:00 - 16:00	19	383	1.072	3.443	19	383	1.320	4.237	19	383	2.392	7.680
16:00 - 17:00	19	383	1.746	5.605	19	383	1.897	6.091	19	383	3.643	11.696
17:00 - 18:00	19	383	2.970	9.533	19	383	3.547	11.387	19	383	6.517	20.920
18:00 - 19:00	18	396	0.225	0.721	18	396	0.870	2.794	18	396	1.095	3.515
19:00 - 20:00	1	400	0.000	0.000	1	400	0.000	0.000	1	400	0.000	0.000
20:00 - 21:00												
21:00 - 22:00												
22:00 - 23:00												
23:00 - 24:00												
Total Rates:			19.145	61.455			18.976	60.920			38.121	122.375

This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: $COUNT/TRP*FACT$. Trip rates are then rounded to 3 decimal places.

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Parameter summary

Trip rate parameter range selected:	150 - 750 (units: sqm)
Survey date date range:	01/01/11 - 12/07/18
Number of weekdays (Monday-Friday):	19
Number of Saturdays:	0
Number of Sundays:	0
Surveys automatically removed from selection:	1
Surveys manually removed from selection:	0

This section displays a quick summary of some of the data filtering selections made by the TRICS® user. The trip rate calculation parameter range of all selected surveys is displayed first, followed by the range of minimum and maximum survey dates selected by the user. Then, the total number of selected weekdays and weekend days in the selected set of surveys are show. Finally, the number of survey days that have been manually removed from the selected set outside of the standard filtering procedure are displayed.

TRIP RATE for Land Use 07 - LEISURE/K - FITNESS CLUB (PRIVATE)
VEHICLES

Calculation factor: 100 sqm

Estimated TRIP rate value per 126 SQM shown in shaded columns

BOLD print indicates peak (busiest) period

Time Range	ARRIVALS				DEPARTURES				TOTALS			
	No. Days	Ave. GFA	Trip Rate	Estimated Trip Rate	No. Days	Ave. GFA	Trip Rate	Estimated Trip Rate	No. Days	Ave. GFA	Trip Rate	Estimated Trip Rate
00:00 - 01:00												
01:00 - 02:00												
02:00 - 03:00												
03:00 - 04:00												
04:00 - 05:00												
05:00 - 06:00												
06:00 - 07:00	16	3753	0.836	1.053	16	3753	0.048	0.061	16	3753	0.884	1.114
07:00 - 08:00	16	3753	0.558	0.703	16	3753	0.566	0.713	16	3753	1.124	1.416
08:00 - 09:00	16	3753	0.841	1.060	16	3753	0.654	0.825	16	3753	1.495	1.885
09:00 - 10:00	16	3753	1.281	1.614	16	3753	0.581	0.732	16	3753	1.862	2.346
10:00 - 11:00	16	3753	0.993	1.251	16	3753	0.831	1.047	16	3753	1.824	2.298
11:00 - 12:00	16	3753	0.603	0.760	16	3753	1.024	1.290	16	3753	1.627	2.050
12:00 - 13:00	16	3753	0.560	0.705	16	3753	0.873	1.100	16	3753	1.433	1.805
13:00 - 14:00	16	3753	0.610	0.768	16	3753	0.754	0.951	16	3753	1.364	1.719
14:00 - 15:00	16	3753	0.580	0.730	16	3753	0.556	0.701	16	3753	1.136	1.431
15:00 - 16:00	16	3753	0.836	1.053	16	3753	0.683	0.860	16	3753	1.519	1.913
16:00 - 17:00	16	3753	1.174	1.479	16	3753	0.856	1.079	16	3753	2.030	2.558
17:00 - 18:00	16	3753	1.684	2.121	16	3753	0.981	1.236	16	3753	2.665	3.357
18:00 - 19:00	16	3753	1.547	1.949	16	3753	1.442	1.817	16	3753	2.989	3.766
19:00 - 20:00	16	3753	1.023	1.288	16	3753	1.540	1.941	16	3753	2.563	3.229
20:00 - 21:00	16	3753	0.545	0.686	16	3753	1.234	1.555	16	3753	1.779	2.241
21:00 - 22:00	15	3443	0.106	0.134	15	3443	0.817	1.030	15	3443	0.923	1.164
22:00 - 23:00	3	2668	0.025	0.031	3	2668	0.250	0.315	3	2668	0.275	0.346
23:00 - 24:00												
Total Rates:			13.802	17.385			13.690	17.253			27.492	34.638

This section displays the trip rate results based on the selected set of surveys and the selected count type (shown just above the table). It is split by three main columns, representing arrivals trips, departures trips, and total trips (arrivals plus departures). Within each of these main columns are three sub-columns. These display the number of survey days where count data is included (per time period), the average value of the selected trip rate calculation parameter (per time period), and the trip rate result (per time period). Total trip rates (the sum of the column) are also displayed at the foot of the table.

*To obtain a trip rate, the average (mean) trip rate parameter value (TRP) is first calculated for all selected survey days that have count data available for the stated time period. The average (mean) number of arrivals, departures or totals (whichever applies) is also calculated (COUNT) for all selected survey days that have count data available for the stated time period. Then, the average count is divided by the average trip rate parameter value, and multiplied by the stated calculation factor (shown just above the table and abbreviated here as FACT). So, the method is: COUNT/TRP*FACT. Trip rates are then rounded to 3 decimal places.*

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Parameter summary

Trip rate parameter range selected:	404 - 9000 (units: sqm)
Survey date date range:	01/01/11 - 27/09/17
Number of weekdays (Monday-Friday):	16
Number of Saturdays:	0
Number of Sundays:	0
Surveys automatically removed from selection:	0
Surveys manually removed from selection:	0

This section displays a quick summary of some of the data filtering selections made by the TRICS® user. The trip rate calculation parameter range of all selected surveys is displayed first, followed by the range of minimum and maximum survey dates selected by the user. Then, the total number of selected weekdays and weekend days in the selected set of surveys are shown. Finally, the number of survey days that have been manually removed from the selected set outside of the standard filtering procedure are displayed.

Appendix 3- Sample Modelling Output File

_____ O S C A D Y 5 _____

Analysis Program: Release 3.0 (Jan 2008)

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For sales and distribution information,
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THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

.Run with file:- "C:\Documents and Settings\obyrrner\Desktop\Northwood\Junction 6 - Santry Ave\J6 2021 AM Peak.voi" at 08:48:21 on
Thursday, 30 May 2019

.FILE PROPERTIES

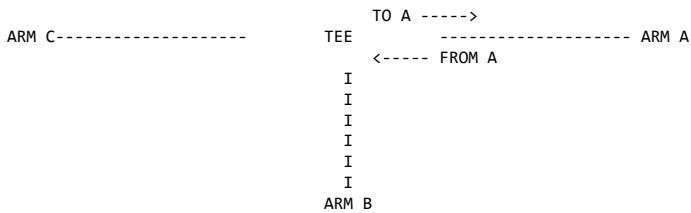
RUN TITLE: Northwood - Junction 6 AM Peak
LOCATION: Northwood
DATE: 29/05/2019
CLIENT: Cosgrove
ENUMERATOR: obyrrner [LIBRARYMONRD]
JOB NUMBER: 19205
STATUS:
DESCRIPTION:

**** ERROR AND WARNING MESSAGES ****
=====

No errors or warnings in the data.

.TRAFFIC SIGNAL JUNCTION ANALYSIS

INPUT DATA



ARM A IS Arm A - Santry Ave WEST
ARM B IS Arm B - Northwood Road
ARM C IS Arm C - Santry Ave EAST

.GEOMETRIC DATA

I	DATA ITEM	I	ARM A	I	ARM B	I	ARM C	I
I	GRADIENT	I	0.0 %	I	0.0 %	I	0.0 %	I
I		I		I		I		I
I	NUMBER OF LANES	I	1	I	1	I	1	I
I		I		I		I		I

J6 2021 AM Peak NoDev.txt

PERMITTED MOVEMENTS	LANE 1	LS	L	R	SR
TOTAL EXIT WIDTH FOR STRAIGHT-AHEAD VEHICLES FROM THIS ARM	N/A	N/A	N/A	N/A	N/A
LANE WIDTHS	LANE 1	3.00 M	3.50 M	3.30 M	
LEFT TURN RADII	LANE 1	15.0 M	20.0 M	N/A	
RIGHT TURN RADII	LANE 1	N/A	20.0 M	20.0 M	
OPPOSING TRAFFIC MOVEMENTS				STRAIGHT	
STORAGE BEYOND STOPLINE	LANE 1	0.0 VEHS	0.0 VEHS	3.0 VEHS	

EXIT WIDTH FOR IMAGINARY ARM D = 50.10

FLARES	ADJACENT LANE	STORAGE (PCU)	RATIO SF THIS BAY /SF ADJACENT LANE
ARM A; BAY 1	1	8	1.00
ARM B; BAY 1	1	5	1.00
ARM C; BAY 1	1	4	1.00

.TRAFFIC DEMAND DATA

DEMAND PROFILES ARE SYNTHESISED USING THE ** ODTAB ** OPTION

DEMAND DATA SUPPLIED BETWEEN TIMES - 07.45 TO 09.15
 PERIOD OF INTEREST (FOR QUEUE AND DELAY CALCULATIONS) - 08.00 TO 09.00

THE FOLLOWING DATA HAS BEEN INPUT

TRAFFIC SCALING FACTOR HAS BEEN SET TO 100 %

Northwood - Junction 2

FROM/TO	ARM A	ARM B	ARM C
ARM A	0.0	138.0	461.0
ARM B	60.0	0.0	149.0
ARM C	426.0	125.0	0.0

TIME PERIOD	ARM	CARS AND LIGHT	MEDIUM GOODS	HEAVY GOODS	BUSES AND COACHES	MOTOR CYCLES	PEDAL CYCLES
08.00-09.00	A	0.990	0.000	0.010	0.000	0.000	0.000
	B	0.990	0.010	0.000	0.000	0.000	0.000
	C	0.960	0.020	0.020	0.000	0.000	0.000

.DATA DETERMINED FOR USE IN SYNTHESIS OF DEMAND PROFILES ARE AS FOLLOWS-

ENTRY/EXIT FLOWS	ARM	TIME WHEN FLOW STARTS TO RISE	TIME WHEN TOP OF PEAK IS REACHED	TIME WHEN FLOW STOPS FALLING	RATE OF FLOW (VEH/MIN) BEFORE PEAK	RATE OF FLOW (VEH/MIN) AT TOP OF PEAK	RATE OF FLOW (VEH/MIN) AFTER PEAK
ENTRY	A	08.00	08.30	09.00	7.49	11.23	7.49
	B	08.00	08.30	09.00	2.61	3.92	2.61
	C	08.00	08.30	09.00	6.89	10.33	6.89

.SIGNAL TIMING DETAILS FOR SIGNAL SET 1

TIMING OPTION- VEHICLE ACTUATED MODE

MAXIMUM CYCLE TIME- 120.0 SECONDS

GLOBAL EFFECTIVE GREEN DISPLACEMENTS - START = 1.4
 END = 2.9

I DATA ITEM	I STAGE 1	I STAGE 2	I STAGE 3	I STAGE 4
I LANES ON GREEN: ARM A	I 1	I	I	I
I B	I	I	I 1	I
I C	I 1	I 1	I	I
I MINIMUM GREEN TIME (SECS)	I 5.0	I 10.0	I 5.0	I 10.0
I PRECEDING INTERSTAGE	I 5.0	I 5.0	I 5.0	I 5.0

DEMAND AND CAPACITY INFORMATION FOR EACH 15 MINUTE TIME SEGMENT BETWEEN 08.00 AND 09.00

I TIME	I MOVEMENT	I DEMAND (VEHS/MIN)	I SAT FLOW (PCU/HR)	I SAT FLOW (VEHS/MIN)	I EFFECTIVE TRUE (SECS)	I GREEN-TIME FLARE+NOTIONL (SECS)	I CAPACITY (VEHS /MIN)
I 08.00-08.15							
I A 1	L S	8.94	1871.9	30.80	52.3	68.9	21.39
I B 1	L R	3.12	1827.9	30.31	9.8	19.6	6.00
I C 1	S R	8.22	1554.9	25.01	67.3	75.7	19.10
I 08.15-08.30							
I A 1	L S	10.95	1871.9	30.80	59.1	75.7	21.42
I B 1	L R	3.82	1827.9	30.31	12.7	22.5	6.27
I C 1	S R	10.07	1493.7	24.03	74.1	82.5	18.23
I 08.30-08.45							
I A 1	L S	10.95	1871.9	30.80	59.1	75.7	21.42
I B 1	L R	3.82	1827.9	30.31	12.7	22.5	6.27
I C 1	S R	10.07	1493.7	24.03	74.1	82.5	18.23
I 08.45-09.00							
I A 1	L S	8.94	1871.9	30.80	52.3	68.9	21.39
I B 1	L R	3.12	1827.9	30.31	9.8	19.6	6.00
I C 1	S R	8.22	1554.9	25.01	67.3	75.7	19.10

QUEUE AND DELAY INFORMATION FOR EACH 15 MINUTE TIME SEGMENT BETWEEN 08.00 AND 09.00

I TIME	I MOVEMENT	I DEMAND EXCL 2-WHEEL (VEHS/MIN)	I CAPACITY (VEHS/MIN)	I DEGREE OF SAT (RFC)	I QUEUE AT END OF SEGMENT MEAN (PHASE AVERAGED) (VEHS/LANE)	I QUEUEING DELAY (VEH.MIN/ TIME SEGMENT)	I GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)
I 08.00-08.15							
I A 1	L S	8.94	21.39	0.418	2.3	7.2	34.4
I B 1	L R	3.12	6.00	0.520	2.5	5.0	37.9
I C 1	S R	8.22	19.10	0.430	1.2	4.6	17.7
I 08.15-08.30							
I A 1	L S	10.95	21.42	0.511	3.2	9.4	47.8
I B 1	L R	3.82	6.27	0.609	3.5	6.7	52.0
I C 1	S R	10.07	18.23	0.553	1.9	6.2	28.4
I 08.30-08.45							
I A 1	L S	10.95	21.42	0.511	3.2	9.4	47.8
I B 1	L R	3.82	6.27	0.609	3.5	6.7	52.2
I C 1	S R	10.07	18.23	0.553	1.9	6.2	28.4
I 08.45-09.00							
I A 1	L S	8.94	21.39	0.418	2.3	7.2	34.4
I B 1	L R	3.12	6.00	0.520	2.5	5.0	38.3
I C 1	S R	8.22	19.10	0.430	1.2	4.6	17.8

* Note * You have specified a flare or bay in which the storage is too large to fully discharge over the green time for the approach. The NEEG has been reduced to allow for this. The affected bay is:-
 BAY 1; on arm B; and it is Adjacent to lane 1

QUEUES FOR ARM A

TIME SEGMENT ENDING	LANE	NUMBER OF MEAN (PHASE AVERAGED) *	VEHICLES IN QUEUE MAXIMUM (AT END OF RED) +	
08.15	1	2.3	7.2	**+++++
08.30	1	3.2	9.4	***+++++
08.45	1	3.2	9.4	***+++++
09.00	1	2.3	7.2	**+++++

.QUEUES FOR ARM B

TIME SEGMENT ENDING	LANE	NUMBER OF MEAN (PHASE AVERAGED) *	VEHICLES IN QUEUE MAXIMUM (AT END OF RED) +	
08.15	1	2.5	5.0	****+
08.30	1	3.5	6.7	***++++
08.45	1	3.5	6.7	***++++
09.00	1	2.5	5.0	****+

.QUEUES FOR ARM C

TIME SEGMENT ENDING	LANE	NUMBER OF MEAN (PHASE AVERAGED) *	VEHICLES IN QUEUE MAXIMUM (AT END OF RED) +	
08.15	1	1.2	4.6	*++++
08.30	1	1.9	6.2	**++++
08.45	1	1.9	6.2	**++++
09.00	1	1.2	4.6	*++++

.QUEUEING DELAY INFORMATION OVER WHOLE PERIOD (08.00-09.00)

I	STREAM	I	TOTAL DEMAND (EXCL 2-WHEEL) (VEH)	I	137.5	I	137.5	I	37.9	I	0.28	I	37.9	I	0.28	I
I		I	(VEH/H)	I	459.3	I	459.3	I	126.5	I	0.28	I	126.6	I	0.28	I
I		I	(VEH)	I	148.4	I	148.4	I	128.6	I	0.87	I	129.0	I	0.87	I
I		I	(VEH)	I	59.8	I	59.8	I	51.8	I	0.87	I	51.9	I	0.87	I
I		I	(VEH)	I	424.4	I	424.4	I	71.4	I	0.17	I	71.4	I	0.17	I
I		I	(VEH)	I	124.5	I	124.5	I	21.0	I	0.17	I	21.0	I	0.17	I
I	ALL	I	1353.9	I	1353.9	I	437.1	I	0.32	I	437.8	I	0.32	I		I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL BE SIGNIFICANTLY DIFFERENT ONLY IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

* TOTAL GEOMETRIC DELAY INCLUDES DELAY SUFFERED BY VEHICLES STILL QUEUEING AT THE END OF THE WHOLE TIME PERIOD.
 * THE SUM OF DELAYS FOR EACH SEGMENT AND THE TOTAL GEOMETRIC DELAY WILL BE SIGNIFICANTLY DIFFERENT ONLY IF THERE IS
 * A LARGE QUEUE AT THE END OF THE TIME PERIOD.

***** OSCADY 5 run completed

===== end of file =====

OSCADY 5

Analysis Program: Release 3.0 (Jan 2008)

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.Run with file:- "C:\Documents and Settings\obyrrner\Desktop\Northwood\Junction 6 - Santry Ave\J6 2021 AM Peak WITHDev.voi" at 08:51:25 on Thursday, 30 May 2019

.FILE PROPERTIES *****

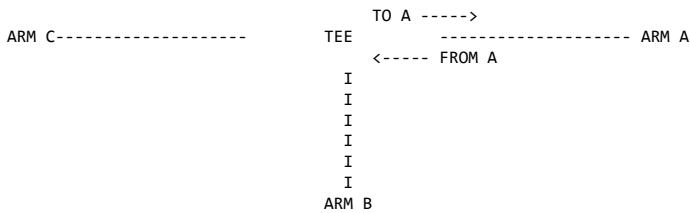
RUN TITLE: Northwood - Junction 6 AM Peak
LOCATION: Northwood
DATE: 29/05/2019
CLIENT: Cosgrove
ENUMERATOR: obyrrner [LIBRARYMONRD]
JOB NUMBER: 19205
STATUS:
DESCRIPTION:

**** ERROR AND WARNING MESSAGES ****
=====

No errors or warnings in the data.

.TRAFFIC SIGNAL JUNCTION ANALYSIS *****

INPUT DATA



ARM A IS Arm A - Santry Ave WEST
ARM B IS Arm B - Northwood Road
ARM C IS Arm C - Santry Ave EAST

.GEOMETRIC DATA

Table with 8 columns: I, DATA ITEM, I, ARM A, I, ARM B, I, ARM C, I. Rows include GRADIENT (0.0%) and NUMBER OF LANES (1).

I	PERMITTED MOVEMENTS	LANE 1	I	LS	I	L R	I	SR	I
I			I		I		I		I
I	TOTAL EXIT WIDTH FOR STRAIGHT-		I		I		I		I
I	AHEAD VEHICLES FROM THIS ARM		I	N/A	I	N/A	I	N/A	I
I			I		I		I		I
I	LANE WIDTHS	LANE 1	I	3.00 M	I	3.50 M	I	3.30 M	I
I			I		I		I		I
I	LEFT TURN RADII	LANE 1	I	15.0 M	I	20.0 M	I	N/A	I
I			I		I		I		I
I	RIGHT TURN RADII	LANE 1	I	N/A	I	20.0 M	I	20.0 M	I
I			I		I		I		I
I	OPPOSING TRAFFIC MOVEMENTS		I		I		I	STRAIGHT	I
I			I		I		I		I
I	STORAGE BEYOND	LANE 1	I	0.0 VEHS	I	0.0 VEHS	I	3.0 VEHS	I
I	STOPLINE		I		I		I		I

EXIT WIDTH FOR IMAGINARY ARM D = 50.10

I	FLARES	I	ADJACENT LANE	I	STORAGE (PCU)	I	RATIO SF THIS BAY	I
I	~~~~~	I		I		I	/SF ADJACENT LANE	I
I	ARM A; BAY 1	I	1	I	8	I	1.00	I
I	ARM B; BAY 1	I	1	I	5	I	1.00	I
I	ARM C; BAY 1	I	1	I	4	I	1.00	I

.TRAFFIC DEMAND DATA

DEMAND PROFILES ARE SYNTHESISED USING THE ** ODTAB ** OPTION

DEMAND DATA SUPPLIED BETWEEN TIMES - 07.45 TO 09.15
 PERIOD OF INTEREST (FOR QUEUE AND DELAY CALCULATIONS) - 08.00 TO 09.00

THE FOLLOWING DATA HAS BEEN INPUT

TRAFFIC SCALING FACTOR HAS BEEN SET TO 100 %

Northwood - Junction 2

I		I	TOTAL TRAFFIC DEMAND (VEHICLES / HOUR)	I
I	FROM/TO	I	ARM A ARM B ARM C	I
I	ARM A	I	0.0 144.0 461.0	I
I	ARM B	I	66.0 0.0 164.0	I
I	ARM C	I	426.0 132.0 0.0	I

I	TIME PERIOD	I	ARM	I	VEHICLE TYPE PROPORTIONS	I
I		I		I	CARS AND MEDIUM HEAVY BUSES AND MOTOR PEDAL	I
I		I		I	LIGHT GOODS GOODS GOODS COACHES CYCLES CYCLES	I
I	08.00-09.00	I	A	I	0.990 0.000 0.010 0.000 0.000 0.000	I
I		I	B	I	0.990 0.010 0.000 0.000 0.000 0.000	I
I		I	C	I	0.960 0.020 0.020 0.000 0.000 0.000	I

.DATA DETERMINED FOR USE IN SYNTHESIS OF DEMAND PROFILES ARE AS FOLLOWS-

I	ENTRY/EXIT FLOWS	I	ARM	I	TIME WHEN FLOW STARTS TO RISE	I	TIME WHEN TOP OF PEAK IS REACHED	I	TIME WHEN FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK	I
I	ENTRY	I	A	I	08.00	I	08.30	I	09.00	I	7.56	I	11.34	I	7.56	I
I		I	B	I	08.00	I	08.30	I	09.00	I	2.88	I	4.31	I	2.88	I
I		I	C	I	08.00	I	08.30	I	09.00	I	6.97	I	10.46	I	6.97	I

.SIGNAL TIMING DETAILS FOR SIGNAL SET 1

TIMING OPTION- VEHICLE ACTUATED MODE

MAXIMUM CYCLE TIME- 120.0 SECONDS

GLOBAL EFFECTIVE GREEN DISPLACEMENTS - START = 1.4
 END = 2.9

DATA ITEM	STAGE 1	STAGE 2	STAGE 3	STAGE 4
LANES ON GREEN: ARM A	1			
B			1	
C	1	1		
MINIMUM GREEN TIME (SECS)	5.0	10.0	5.0	10.0
PRECEDING INTERSTAGE	5.0	5.0	5.0	5.0

DEMAND AND CAPACITY INFORMATION FOR EACH 15 MINUTE TIME SEGMENT BETWEEN 08.00 AND 09.00

TIME	MOVEMENT	DEMAND (VEHS/MIN)	SAT FLOW (PCU/HR)	SAT FLOW (VEHS/MIN)	EFFECTIVE TRUE (SECS)	GREEN-TIME FLARE+NOTIONL (SECS)	CAPACITY (VEHS /MIN)
08.00-08.15							
A 1	L S	9.03	1870.5	30.77	49.2	65.7	20.92
B 1	L R	3.43	1827.9	30.31	10.5	20.4	6.38
C 1	S R	8.33	1544.4	24.85	64.2	72.6	18.65
08.15-08.30							
A 1	L S	11.06	1870.5	30.77	56.4	72.9	20.96
B 1	L R	4.20	1827.9	30.31	13.7	23.6	6.67
C 1	S R	10.20	1479.0	23.79	71.4	79.8	17.72
08.30-08.45							
A 1	L S	11.06	1870.5	30.77	56.4	72.9	20.96
B 1	L R	4.20	1827.9	30.31	13.7	23.6	6.67
C 1	S R	10.20	1479.0	23.79	71.4	79.8	17.72
08.45-09.00							
A 1	L S	9.03	1870.5	30.77	49.2	65.7	20.92
B 1	L R	3.43	1827.9	30.31	10.5	20.4	6.38
C 1	S R	8.33	1544.4	24.85	64.2	72.6	18.65

QUEUE AND DELAY INFORMATION FOR EACH 15 MINUTE TIME SEGMENT BETWEEN 08.00 AND 09.00

TIME	MOVEMENT	DEMAND EXCL 2-WHEEL (VEHS/MIN)	CAPACITY (VEHS/MIN)	DEGREE OF SAT (RFC)	QUEUE AT END OF SEGMENT MEAN (PHASE AVERAGED) (VEHS/LANE)	QUEUEING DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)
08.00-08.15							
A 1	L S	9.03	20.92	0.432	2.4	7.3	36.7
B 1	L R	3.43	6.38	0.538	2.7	5.3	40.4
C 1	S R	8.33	18.65	0.447	1.3	4.7	19.4
08.15-08.30							
A 1	L S	11.06	20.96	0.528	3.4	9.7	51.3
B 1	L R	4.20	6.67	0.630	3.7	7.2	55.9
C 1	S R	10.20	17.72	0.576	2.1	6.5	31.6
08.30-08.45							
A 1	L S	11.06	20.96	0.528	3.4	9.7	51.3
B 1	L R	4.20	6.67	0.630	3.7	7.2	56.2
C 1	S R	10.20	17.72	0.576	2.1	6.5	31.7
08.45-09.00							
A 1	L S	9.03	20.92	0.432	2.4	7.3	36.7
B 1	L R	3.43	6.38	0.538	2.7	5.3	40.8
C 1	S R	8.33	18.65	0.447	1.3	4.7	19.5

QUEUES FOR ARM A

TIME SEGMENT ENDING	LANE	MEAN (PHASE AVERAGED)	NUMBER OF VEHICLES IN QUEUE (AT END OF RED)
---------------------	------	-----------------------	---

	*	+
08.15	1	2.4 7.3 **++++
08.30	1	3.4 9.7 ***+++++
08.45	1	3.4 9.7 ***+++++
09.00	1	2.4 7.3 **++++

.QUEUES FOR ARM B

TIME SEGMENT ENDING	LANE	NUMBER OF MEAN (PHASE AVERAGED)	VEHICLES IN QUEUE (AT END OF RED)	
		*	+	
08.15	1	2.7	5.3	****+
08.30	1	3.7	7.2	*****
08.45	1	3.7	7.2	*****
09.00	1	2.7	5.3	****+

.QUEUES FOR ARM C

TIME SEGMENT ENDING	LANE	NUMBER OF MEAN (PHASE AVERAGED)	VEHICLES IN QUEUE (AT END OF RED)	
		*	+	
08.15	1	1.3	4.7	*****
08.30	1	2.1	6.5	*****
08.45	1	2.1	6.5	*****
09.00	1	1.3	4.7	*****

.QUEUEING DELAY INFORMATION OVER WHOLE PERIOD (08.00-09.00)

I STREAM I	I TOTAL DEMAND I	I (EXCL 2-WHEEL) I	I * QUEUEING * I	I * DELAY * I	I * INCLUSIVE QUEUEING * I	I * DELAY * I
I	I (VEH)	I (VEH/H)	I (MIN)	I (MIN/VEH)	I (MIN)	I (MIN/VEH)
I A-B I	143.5 I	143.5 I	41.9 I	0.29 I	41.9 I	0.29 I
I A-C I	459.3 I	459.3 I	134.1 I	0.29 I	134.2 I	0.29 I
I B-C I	163.4 I	163.4 I	137.9 I	0.84 I	138.3 I	0.85 I
I B-A I	65.7 I	65.7 I	55.5 I	0.84 I	55.6 I	0.85 I
I C-A I	424.4 I	424.4 I	77.9 I	0.18 I	78.0 I	0.18 I
I C-B I	131.5 I	131.5 I	24.2 I	0.18 I	24.2 I	0.18 I
I ALL I	1387.7 I	1387.7 I	471.4 I	0.34 I	472.2 I	0.34 I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL BE SIGNIFICANTLY DIFFERENT ONLY IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

* TOTAL GEOMETRIC DELAY INCLUDES DELAY SUFFERED BY VEHICLES STILL QUEUEING AT THE END OF THE WHOLE TIME PERIOD.
 * THE SUM OF DELAYS FOR EACH SEGMENT AND THE TOTAL GEOMETRIC DELAY WILL BE SIGNIFICANTLY DIFFERENT ONLY IF THERE IS
 * A LARGE QUEUE AT THE END OF THE TIME PERIOD.

***** OSCADY 5 run completed
 ===== end of file =====

_____ O S C A D Y 5 _____

Analysis Program: Release 3.0 (Jan 2008)

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IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

.Run with file:- "C:\Documents and Settings\obyrner\Desktop\Northwood\Junction 7 Swords Road\J7 2021 AM Peak NoDev.voi" at 09:18:12 on
Thursday, 30 May 2019

.FILE PROPERTIES

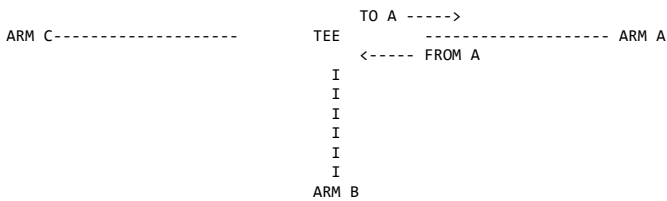
RUN TITLE: Northwood - Junction 7 AM Peak
LOCATION: Northwood
DATE: 29/05/2019
CLIENT: Cosgrove
ENUMERATOR: obyrner [LIBRARYMONRD]
JOB NUMBER: 19205
STATUS:
DESCRIPTION:

**** ERROR AND WARNING MESSAGES ****
=====

No errors or warnings in the data.

.TRAFFIC SIGNAL JUNCTION ANALYSIS

INPUT DATA



ARM A IS Arm A - Swords Road South
ARM B IS Arm B - Northwood Avenue
ARM C IS Arm C - Swords Road North

.GEOMETRIC DATA

I	DATA ITEM	I	ARM A	I	ARM B	I	ARM C	I
I	GRADIENT	I	0.0 %	I	0.0 %	I	0.0 %	I
I		I		I		I		I
I	NUMBER OF LANES	I	2	I	1	I	1	I
I		I		I		I		I
I	PERMITTED MOVEMENTS LANE 1	I	L	I	L R	I	SR	I
I		I	S	I		I		I
I		I		I		I		I
I	TOTAL EXIT WIDTH FOR STRAIGHT-	I		I		I		I
I	AHEAD VEHICLES FROM THIS ARM	I	N/A	I	N/A	I	N/A	I
I		I		I		I		I

I	LANE WIDTHS	LANE 1	I	3.00 M	I	3.50 M	I	3.00 M	I
I		LANE 2	I	3.00 M	I	0.00 M	I	0.00 M	I
I			I		I		I		I
I	LEFT TURN RADII	LANE 1	I	9.0 M	I	20.0 M	I	N/A	I
I			I		I		I		I
I	RIGHT TURN RADII	LANE 1	I	N/A	I	20.0 M	I	20.0 M	I
I			I		I		I		I
I	OPPOSING TRAFFIC MOVEMENTS		I		I		I	STRAIGHT	I
I			I		I		I		I
I	STORAGE BEYOND STOPLINE	LANE 1	I	0.0 VEHS	I	0.0 VEHS	I	2.0 VEHS	I
I			I		I		I		I

EXIT WIDTH FOR IMAGINARY ARM D = 50.10

I	FLARES	I	ADJACENT LANE	I	STORAGE (PCU)	I	RATIO SF THIS BAY	I
I	~~~~~	I		I		I	/SF ADJACENT LANE	I
I	ARM B; BAY 1	I	1	I	6	I	1.00	I
I	ARM C; BAY 1	I	1	I	6	I	1.00	I

.TRAFFIC DEMAND DATA

DEMAND PROFILES ARE SYNTHESISED USING THE ** ODTAB ** OPTION

DEMAND DATA SUPPLIED BETWEEN TIMES - 07.45 TO 09.15
 PERIOD OF INTEREST (FOR QUEUE AND DELAY CALCULATIONS) - 08.00 TO 09.00

THE FOLLOWING DATA HAS BEEN INPUT

TRAFFIC SCALING FACTOR HAS BEEN SET TO 100 %

Northwood - Junction 2

I		I	TOTAL TRAFFIC DEMAND (VEHICLES / HOUR)	I
I	FROM/TO	I	ARM A ARM B ARM C	I
I	ARM A	I	0.0 126.0 465.0	I
I	ARM B	I	218.0 0.0 220.0	I
I	ARM C	I	485.0 165.0 0.0	I

I	TIME PERIOD	I	ARM	I	VEHICLE TYPE PROPORTIONS	I
I		I		I	CARS AND MEDIUM HEAVY BUSES AND MOTOR PEDAL	I
I		I		I	LIGHT GOODS GOODS GOODS COACHES CYCLES CYCLES	I
I	08.00-09.00	I	A	I	0.990 0.010 0.000 0.000 0.000 0.000	I
I		I	B	I	0.980 0.010 0.010 0.000 0.000 0.000	I
I		I	C	I	0.990 0.010 0.000 0.000 0.000 0.000	I

.DATA DETERMINED FOR USE IN SYNTHESIS OF DEMAND PROFILES ARE AS FOLLOWS-

I	ENTRY/EXIT FLOWS	I	ARM	I	TIME WHEN FLOW STARTS TO RISE	I	TIME WHEN TOP OF PEAK IS REACHED	I	TIME WHEN FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	RATE OF FLOW (VEH/MIN) AT TOP OF PEAK	I	RATE OF FLOW (VEH/MIN) AFTER PEAK	I
I	ENTRY	I	A	I	08.00	I	08.30	I	09.00	I	7.39	I	11.08	I	7.39	I
I		I	B	I	08.00	I	08.30	I	09.00	I	5.47	I	8.21	I	5.47	I
I		I	C	I	08.00	I	08.30	I	09.00	I	8.13	I	12.19	I	8.13	I

.SIGNAL TIMING DETAILS FOR SIGNAL SET 1

TIMING OPTION- VEHICLE ACTUATED MODE

MAXIMUM CYCLE TIME- 120.0 SECONDS

GLOBAL EFFECTIVE GREEN DISPLACEMENTS - START = 1.4
 END = 2.9

I	DATA ITEM	I	STAGE 1	I	STAGE 2	I	STAGE 3	I	STAGE 4	I
I	LANES ON GREEN: ARM A	I	1 2	I		I		I		I
I		I	B	I		I	1	I		I

I	C	I 1	I 1	I	I	I
I		I	I	I	I	I
I	MINIMUM GREEN TIME (SECS)	I 10.0	I 10.0	I 15.0	I 10.0	I
I		I	I	I	I	I
I	PRECEDING INTERSTAGE	I 5.0	I 5.0	I 5.0	I 5.0	I

.DEMAND AND CAPACITY INFORMATION FOR EACH 15 MINUTE TIME SEGMENT BETWEEN 08.00 AND 09.00

I	TIME	MOVEMENT	DEMAND (VEHS/MIN)	SAT FLOW (PCU/HR)	SAT FLOW (VEHS/MIN)	EFFECTIVE TRUE (SECS)	GREEN-TIME FLARE+NOTIONL (SECS)	CAPACITY (VEHS /MIN)	I
I	ARM	LANES							I

I	08.00-08.15								I
I	A 1	L	1.88	1641.4	27.22	35.3		10.68	I
I	2	S	6.94	2055.0	34.08	35.3		13.37	I
I	B 1	L R	6.54	1827.9	29.93	17.7	29.5	9.80	I
I	C 1	S R	9.70	1302.3	21.60	50.3	63.1	15.15	I

I	08.15-08.30								I
I	A 1	L	2.30	1641.4	27.22	55.1		12.50	I
I	2	S	8.50	2055.0	34.08	55.1		15.65	I
I	B 1	L R	8.01	1827.9	29.93	27.9	39.7	9.90	I
I	C 1	S R	11.88	1234.7	20.48	70.1	82.9	14.15	I

I	08.30-08.45								I
I	A 1	L	2.30	1641.4	27.22	55.1		12.50	I
I	2	S	8.50	2055.0	34.08	55.1		15.65	I
I	B 1	L R	8.01	1827.9	29.93	27.9	39.7	9.90	I
I	C 1	S R	11.88	1234.7	20.48	70.1	82.9	14.15	I

I	08.45-09.00								I
I	A 1	L	1.88	1641.4	27.22	35.3		10.68	I
I	2	S	6.94	2055.0	34.08	35.3		13.37	I
I	B 1	L R	6.54	1827.9	29.93	17.7	29.5	9.80	I
I	C 1	S R	9.70	1302.3	21.60	50.3	63.1	15.15	I

.QUEUE AND DELAY INFORMATION FOR EACH 15 MINUTE TIME SEGMENT BETWEEN 08.00 AND 09.00

I	TIME	MOVEMENT	DEMAND EXCL 2-WHEEL (VEHS/MIN)	CAPACITY (VEHS/MIN)	DEGREE OF SAT (RFC)	QUEUE AT END OF SEGMENT MEAN (PHASE AVERAGED) (VEHS/LANE)	MAXIMUM (END OF RED) (VEHS/LANE)	QUEUEING DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	I
I	ARM	LANES								I

I	08.00-08.15									I
I	A 1	L	1.88	10.68	0.176	0.6	1.7	8.7		I
I	2	S	6.94	13.37	0.519	2.7	6.7	41.1		I
I	B 1	L R	6.54	9.80	0.667	4.4	8.7	65.9		I
I	C 1	S R	9.70	15.15	0.640	2.9	7.1	42.9		I

I	08.15-08.30									I
I	A 1	L	2.30	12.50	0.184	0.8	2.5	11.4		I
I	2	S	8.50	15.65	0.543	3.7	9.6	55.5		I
I	B 1	L R	8.01	9.90	0.809	7.7	14.2	114.8		I
I	C 1	S R	11.88	14.15	0.840	6.4	12.3	95.2		I

I	08.30-08.45									I
I	A 1	L	2.30	12.50	0.184	0.8	2.5	11.4		I
I	2	S	8.50	15.65	0.543	3.7	9.6	55.5		I
I	B 1	L R	8.01	9.90	0.809	7.8	14.2	117.5		I
I	C 1	S R	11.88	14.15	0.840	6.5	12.4	99.4		I

I	08.45-09.00									I
I	A 1	L	1.88	10.68	0.176	0.6	1.7	8.7		I
I	2	S	6.94	13.37	0.519	2.8	6.7	41.2		I
I	B 1	L R	6.54	9.80	0.667	4.5	8.7	67.8		I
I	C 1	S R	9.70	15.15	0.640	2.9	7.1	44.3		I

.QUEUES FOR ARM A

TIME SEGMENT ENDING	LANE	NUMBER OF VEHICLES IN QUEUE		
		MEAN (PHASE AVERAGED)	MAXIMUM (AT END OF RED)	
		*	+	
08.15	2	2.7	6.7	*****
	1	0.6	1.7	**
08.30	2	3.7	9.6	*****
	1	0.8	2.5	**
08.45	2	3.7	9.6	*****

	1	0.8	2.5	*++
09.00	2	2.8	6.7	***++++
	1	0.6	1.7	*+

.QUEUES FOR ARM B

```

-----
TIME    LANE    NUMBER OF VEHICLES IN QUEUE
SEGMENT LANE    MEAN    MAXIMUM
ENDING  ENDING  (PHASE  (AT END
          AVERAGED) OF RED)
          *      +
08.15   1      4.4     8.7    ****++++
08.30   1      7.7    14.2   *****++++
08.45   1      7.8    14.2   *****++++
09.00   1      4.5     8.7    ****++++
    
```

.QUEUES FOR ARM C

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-----
TIME    LANE    NUMBER OF VEHICLES IN QUEUE
SEGMENT LANE    MEAN    MAXIMUM
ENDING  ENDING  (PHASE  (AT END
          AVERAGED) OF RED)
          *      +
08.15   1      2.9     7.1    ***++++
08.30   1      6.4    12.3   *****++++
08.45   1      6.5    12.4   *****++++
09.00   1      2.9     7.1    ***++++
    
```

.QUEUEING DELAY INFORMATION OVER WHOLE PERIOD (08.00-09.00)

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-----
I STREAM I  TOTAL DEMAND I  * QUEUEING *  I  * INCLUSIVE QUEUEING * I
I         I (EXCL 2-WHEEL) I  * DELAY *  I  * DELAY *  I
I         I         I         I         I         I         I
I         I (VEH) (VEH/H) I (MIN) (MIN/VEH) I (MIN) (MIN/VEH) I
-----
I A-B I 125.5 I 125.5 I 40.3 I 0.32 I 40.3 I 0.32 I
I A-C I 463.2 I 463.2 I 193.3 I 0.42 I 193.6 I 0.42 I
I B-C I 219.2 I 219.2 I 183.8 I 0.84 I 184.3 I 0.84 I
I B-A I 217.2 I 217.2 I 182.1 I 0.84 I 182.6 I 0.84 I
I C-A I 483.2 I 483.2 I 210.3 I 0.44 I 210.5 I 0.44 I
I C-B I 164.4 I 164.4 I 71.5 I 0.44 I 71.6 I 0.44 I
-----
I ALL I 1672.6 I 1672.6 I 881.3 I 0.53 I 882.9 I 0.53 I
-----
    
```

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL BE SIGNIFICANTLY DIFFERENT ONLY IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

* TOTAL GEOMETRIC DELAY INCLUDES DELAY SUFFERED BY VEHICLES STILL QUEUEING AT THE END OF THE WHOLE TIME PERIOD.
 * THE SUM OF DELAYS FOR EACH SEGMENT AND THE TOTAL GEOMETRIC DELAY WILL BE SIGNIFICANTLY DIFFERENT ONLY IF THERE IS
 * A LARGE QUEUE AT THE END OF THE TIME PERIOD.

***** OSCADY 5 run completed

===== end of file =====

_____ O S C A D Y 5 _____

Analysis Program: Release 3.0 (Jan 2008)

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THE USER OF THIS COMPUTER PROGRAM FOR THE SOLUTION OF AN ENGINEERING PROBLEM IS
IN NO WAY RELIEVED OF THEIR RESPONSIBILITY FOR THE CORRECTNESS OF THE SOLUTION

.Run with file:- "C:\Documents and Settings\obyerner\Desktop\Northwood\Junction 7 Swords Road\J7 2021 AM Peak WITHDev.voi" at 09:20:04 on
Thursday, 30 May 2019

.FILE PROPERTIES

RUN TITLE: Northwood - Junction 7 AM Peak
LOCATION: Northwood
DATE: 29/05/2019
CLIENT: Cosgrove
ENUMERATOR: obyerner [LIBRARYMONRD]
JOB NUMBER: 19205
STATUS:
DESCRIPTION:

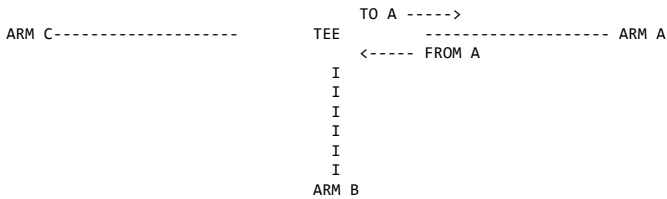
**** ERROR AND WARNING MESSAGES ****

=====

No errors or warnings in the data.

.TRAFFIC SIGNAL JUNCTION ANALYSIS

INPUT DATA



ARM A IS Arm A - Swords Road South
ARM B IS Arm B - Northwood Avenue
ARM C IS Arm C - Swords Road North

.GEOMETRIC DATA

I	DATA ITEM	I	ARM A	I	ARM B	I	ARM C	I
I	GRADIENT	I	0.0 %	I	0.0 %	I	0.0 %	I
I		I		I		I		I
I	NUMBER OF LANES	I	2	I	1	I	1	I
I		I		I		I		I
I	PERMITTED MOVEMENTS LANE 1	I	L	I	L R	I	SR	I
I	LANE 2	I	S	I		I		I
I		I		I		I		I
I	TOTAL EXIT WIDTH FOR STRAIGHT-	I		I		I		I
I	AHEAD VEHICLES FROM THIS ARM	I	N/A	I	N/A	I	N/A	I
I		I		I		I		I

I	LANE WIDTHS	LANE 1	I	3.00 M	I	3.50 M	I	3.00 M	I
I		LANE 2	I	3.00 M	I	0.00 M	I	0.00 M	I
I			I		I		I		I
I	LEFT TURN RADII	LANE 1	I	9.0 M	I	20.0 M	I	N/A	I
I			I		I		I		I
I	RIGHT TURN RADII	LANE 1	I	N/A	I	20.0 M	I	20.0 M	I
I			I		I		I		I
I	OPPOSING TRAFFIC MOVEMENTS		I		I		I	STRAIGHT	I
I			I		I		I		I
I	STORAGE BEYOND STOPLINE	LANE 1	I	0.0 VEHS	I	0.0 VEHS	I	2.0 VEHS	I
I			I		I		I		I

EXIT WIDTH FOR IMAGINARY ARM D = 50.10

I	FLARES	I	ADJACENT LANE	I	STORAGE (PCU)	I	RATIO SF THIS BAY	I
I	~~~~~	I		I		I	/SF ADJACENT LANE	I
I	ARM B; BAY 1	I	1	I	6	I	1.00	I
I	ARM C; BAY 1	I	1	I	6	I	1.00	I

.TRAFFIC DEMAND DATA

DEMAND PROFILES ARE SYNTHESISED USING THE ** ODTAB ** OPTION

DEMAND DATA SUPPLIED BETWEEN TIMES - 07.45 TO 09.15
 PERIOD OF INTEREST (FOR QUEUE AND DELAY CALCULATIONS) - 08.00 TO 09.00

THE FOLLOWING DATA HAS BEEN INPUT

TRAFFIC SCALING FACTOR HAS BEEN SET TO 100 %

Northwood - Junction 2

I		I	TOTAL TRAFFIC DEMAND (VEHICLES / HOUR)	I
I	FROM/TO	I	ARM A ARM B ARM C	I
I	ARM A	I	0.0 182.0 465.0	I
I	ARM B	I	241.0 0.0 243.0	I
I	ARM C	I	485.0 172.0 0.0	I

I	TIME PERIOD	I	ARM	I	VEHICLE TYPE PROPORTIONS	I
I		I		I	CARS AND MEDIUM HEAVY BUSES AND MOTOR PEDAL	I
I		I		I	LIGHT GOODS GOODS GOODS COACHES CYCLES CYCLES	I
I	08.00-09.00	I	A	I	0.990 0.010 0.000 0.000 0.000 0.000	I
I		I	B	I	0.980 0.010 0.010 0.000 0.000 0.000	I
I		I	C	I	0.990 0.010 0.000 0.000 0.000 0.000	I

.DATA DETERMINED FOR USE IN SYNTHESIS OF DEMAND PROFILES ARE AS FOLLOWS-

I	ENTRY/EXIT FLOWS	I	ARM	I	TIME WHEN FLOW STARTS TO RISE	I	TIME WHEN TOP OF PEAK IS REACHED	I	TIME WHEN FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	RATE OF FLOW (VEH/MIN) AT TOP OF PEAK	I	RATE OF FLOW (VEH/MIN) AFTER PEAK	I
I	ENTRY	I	A	I	08.00	I	08.30	I	09.00	I	8.09	I	12.13	I	8.09	I
I		I	B	I	08.00	I	08.30	I	09.00	I	6.05	I	9.08	I	6.05	I
I		I	C	I	08.00	I	08.30	I	09.00	I	8.21	I	12.32	I	8.21	I

.SIGNAL TIMING DETAILS FOR SIGNAL SET 1

TIMING OPTION- VEHICLE ACTUATED MODE

MAXIMUM CYCLE TIME- 120.0 SECONDS

GLOBAL EFFECTIVE GREEN DISPLACEMENTS - START = 1.4
 END = 2.9

I	DATA ITEM	I	STAGE 1	I	STAGE 2	I	STAGE 3	I	STAGE 4	I
I	LANES ON GREEN: ARM A	I	1 2	I		I		I		I
I		I	B	I		I	1	I		I

	C	I 1	I 1	I	I	I
MINIMUM GREEN TIME (SECS)		10.0	10.0	15.0	10.0	
PRECEDING INTERSTAGE		5.0	5.0	5.0	5.0	

.DEMAND AND CAPACITY INFORMATION FOR EACH 15 MINUTE TIME SEGMENT BETWEEN 08.00 AND 09.00

TIME	MOVEMENT	DEMAND (VEHS/MIN)	SAT FLOW (PCU/HR)	SAT FLOW (VEHS/MIN)	EFFECTIVE TRUE (SECS)	GREEN-TIME FLARE+NOTIONL (SECS)	CAPACITY (VEHS /MIN)
I 08.00-08.15							
A 1	L	2.72	1641.4	27.22	35.6		10.45
A 2	S	6.94	2055.0	34.08	35.6		13.08
B 1	L R	7.22	1827.9	29.93	20.1	31.9	10.31
C 1	S R	9.81	1280.3	21.23	50.6	63.4	14.52
I 08.15-08.30							
A 1	L	3.33	1641.4	27.22	53.4		12.12
A 2	S	8.50	2055.0	34.08	53.4		15.18
B 1	L R	8.85	1827.9	29.93	29.6	41.4	10.32
C 1	S R	12.01	1208.2	20.04	68.4	81.3	13.57
I 08.30-08.45							
A 1	L	3.33	1641.4	27.22	53.4		12.12
A 2	S	8.50	2055.0	34.08	53.4		15.18
B 1	L R	8.85	1827.9	29.93	29.6	41.4	10.32
C 1	S R	12.01	1208.2	20.04	68.4	81.3	13.57
I 08.45-09.00							
A 1	L	2.72	1641.4	27.22	35.6		10.45
A 2	S	6.94	2055.0	34.08	35.6		13.08
B 1	L R	7.22	1827.9	29.93	20.1	31.9	10.31
C 1	S R	9.81	1280.3	21.23	50.6	63.4	14.52

.QUEUE AND DELAY INFORMATION FOR EACH 15 MINUTE TIME SEGMENT BETWEEN 08.00 AND 09.00

TIME	MOVEMENT	DEMAND EXCL 2-WHEEL (VEHS/MIN)	CAPACITY (VEHS/MIN)	DEGREE OF SAT (RFC)	QUEUE AT END OF SEGMENT MEAN (PHASE AVERAGED) (VEHS/LANE)	QUEUEING DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)
I 08.00-08.15							
A 1	L	2.72	10.45	0.260	0.9	2.6	14.1
A 2	S	6.94	13.08	0.531	2.9	7.0	43.6
B 1	L R	7.22	10.31	0.701	5.0	9.7	74.2
C 1	S R	9.81	14.52	0.675	3.3	7.7	49.2
I 08.15-08.30							
A 1	L	3.33	12.12	0.274	1.2	3.8	18.4
A 2	S	8.50	15.18	0.560	3.9	9.9	58.7
B 1	L R	8.85	10.32	0.858	9.0	16.0	134.1
C 1	S R	12.01	13.57	0.885	7.9	13.8	115.4
I 08.30-08.45							
A 1	L	3.33	12.12	0.274	1.2	3.8	18.4
A 2	S	8.50	15.18	0.560	3.9	9.9	58.7
B 1	L R	8.85	10.32	0.858	9.2	16.2	140.3
C 1	S R	12.01	13.57	0.885	8.2	14.0	125.7
I 08.45-09.00							
A 1	L	2.72	10.45	0.260	0.9	2.6	14.1
A 2	S	6.94	13.08	0.531	2.9	7.0	43.8
B 1	L R	7.22	10.31	0.701	5.1	9.8	77.5
C 1	S R	9.81	14.52	0.675	3.3	7.7	51.9

.QUEUES FOR ARM A

TIME SEGMENT	LANE	MEAN (PHASE AVERAGED)	MAXIMUM (AT END OF RED)	NUMBER OF VEHICLES IN QUEUE
08.15	2	2.9	7.0	*****
	1	0.9	2.6	***
08.30	2	3.9	9.9	*****
	1	1.2	3.8	***
08.45	2	3.9	9.9	*****

```

        1      1.2      3.8      *+++
09.00   2      2.9      7.0      ***++++
        1      0.9      2.6      **+
```

.QUEUES FOR ARM B

TIME SEGMENT ENDING	LANE	NUMBER OF MEAN (PHASE AVERAGED)	VEHICLES MAXIMUM (AT END OF RED)	IN QUEUE
08.15	1	5.0	9.7	*****+++++
08.30	1	9.0	16.0	*****+++++
08.45	1	9.2	16.2	*****+++++
09.00	1	5.1	9.8	*****+++++

.QUEUES FOR ARM C

TIME SEGMENT ENDING	LANE	NUMBER OF MEAN (PHASE AVERAGED)	VEHICLES MAXIMUM (AT END OF RED)	IN QUEUE
08.15	1	3.3	7.7	***+++++
08.30	1	7.9	13.8	*****+++++
08.45	1	8.2	14.0	*****+++++
09.00	1	3.3	7.7	***+++++

.QUEUEING DELAY INFORMATION OVER WHOLE PERIOD (08.00-09.00)

I	STREAM	I	TOTAL DEMAND (EXCL 2-WHEEL)	I	* QUEUEING * DELAY	I	* INCLUSIVE QUEUEING * DELAY	I
I	I	I	(VEH)	I	(VEH/H)	I	(MIN)	I
I	I	I	(VEH)	I	(VEH/H)	I	(MIN)	I
I	A-B	I	181.3	I	181.3	I	65.0	I
I	A-C	I	463.2	I	463.2	I	204.7	I
I	B-C	I	242.1	I	242.1	I	214.0	I
I	B-A	I	240.1	I	240.1	I	212.2	I
I	C-A	I	483.2	I	483.2	I	252.6	I
I	C-B	I	171.3	I	171.3	I	89.6	I
I	ALL	I	1781.2	I	1781.2	I	1038.1	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD.
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD.
 * THESE WILL BE SIGNIFICANTLY DIFFERENT ONLY IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

* TOTAL GEOMETRIC DELAY INCLUDES DELAY SUFFERED BY VEHICLES STILL QUEUEING AT THE END OF THE WHOLE TIME PERIOD.
 * THE SUM OF DELAYS FOR EACH SEGMENT AND THE TOTAL GEOMETRIC DELAY WILL BE SIGNIFICANTLY DIFFERENT ONLY IF THERE IS
 * A LARGE QUEUE AT THE END OF THE TIME PERIOD.

.***** OSCADY 5 run completed

===== end of file =====